



FIRST WISCONSIN ~ MILWAUKEE BROWN DEER ROAD AT 64TH STREET BRANCH

BROWN DEER, WISCONSIN

PROJECT NUMBER 89214

SHEET INDEX

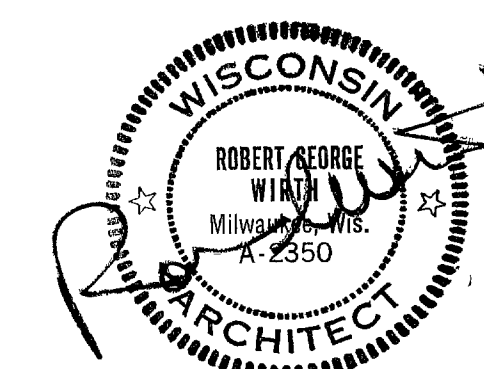
ARCHITECTURAL

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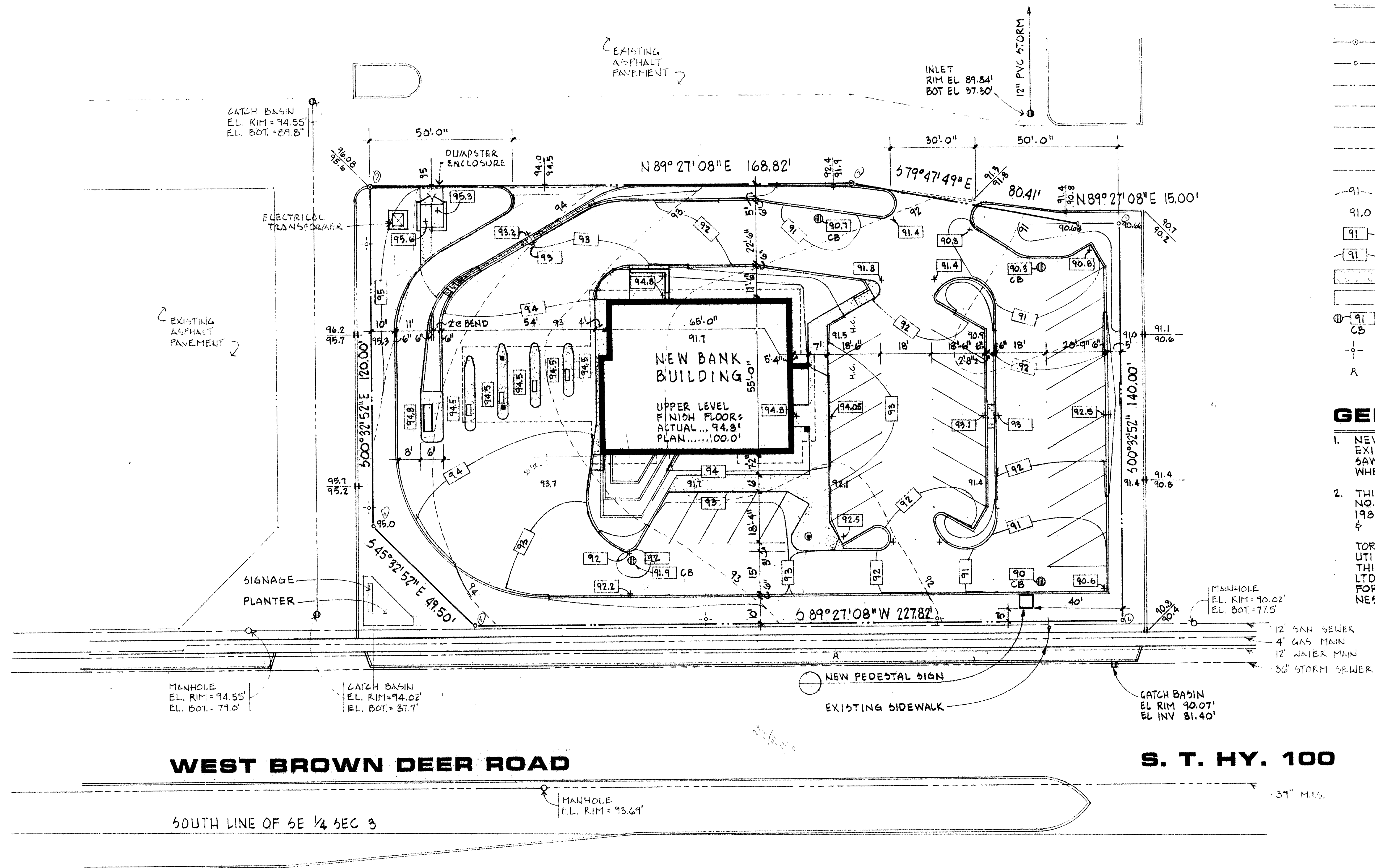
STRUCTURAL

- S1. FOUNDATION AND UPPER LEVEL FRAMING PLAN
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NOV 28 1989
BUILDING PLANS
Conditionally
APPROVED
DEPARTMENT OF INDUSTRY, LABOR AND HUMAN RELATIONS
[Signature]
SEE CORRESPONDENCE
89-11-0197-B



**ARCHITECTURALLY
APPROVED**
Village of Brown Deer
Building Board
Date *12.18.89*



LEGEND

- CROSS IN PAVEMENT
- IRON PIPE
- — — PROPERTY LINE
- — — EXIST SAN SEWER
- — — EXIST GAS MAIN
- — — EXIST WATER MAIN
- — — EXIST STORM SEWER
- 91- EXIST CONTOUR
- 91.0 EXIST SPOT ELEVATION
- [91] NEW SPOT ELEVATION
- 91- NEW CONTOUR
- [] NEW CONCRETE
- [] NEW BITUMINOUS PAVEMENT
- [91] NEW CATCH BASIN w/ RIM ELEVATION
- EXIST POLE LIGHT
- A EXIST HYDRANT

GENERAL NOTES

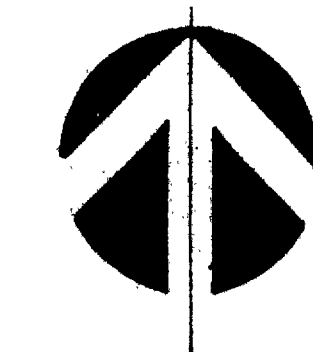
1. NEW PAVEMENT & CURBING TO MATCH EXISTING ALONG NORTH LOT LINE. SAWCUT EXISTING PAVEMENT & CURB WHERE REMOVAL IS REQUIRED.
 2. THIS SITE PLAN IS BASED ON SURVEY NO. 153088-5 DATED SEPTEMBER 29, 1989 PREPARED BY NATIONAL SURVEY & ENGINEERING OF BROOKFIELD, WI.
- TORKE / WIRTH / PUJARA LTD'S DIMENSIONS, UTILITIES & GRADES ARE BASED ON THIS SURVEY. TORKE / WIRTH / PUJARA LTD. ASSUMES NO RESPONSIBILITY FOR THE ACCURACY OR COMPLETENESS OF THE INFORMATION SHOWN.

12" SAN SEWER
4" GAS MAIN
12" WATER MAIN
36" STORM SEWER

S. T. HY. 100

WEST BROWN DEER ROAD

SOUTH LINE OF 6E 1/4 SEC 3



SITE PLAN

SCALE 1"=20'-0"

Rev.	Date	Description	By	Appr.
1	11-6-89	GENEAL	BY	1-6
2	10-30-89		III	Approved
				Date

FIRST WISCONSIN
Brown Deer Road at
64th Street Branch

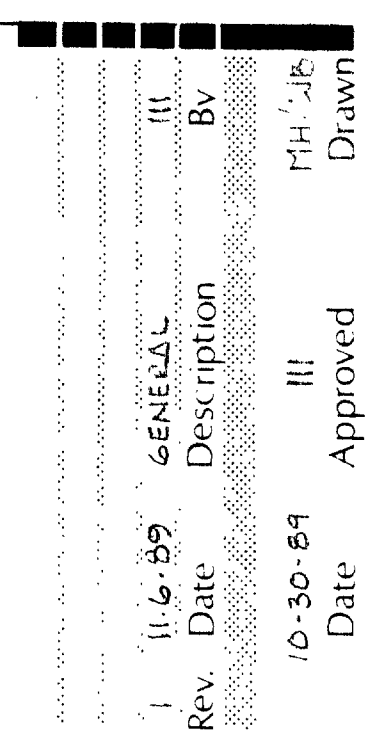
architects engineers
(414) 453-4554

Torke Wirth Pujara Ltd.
933 N. Mayfair Road Wauwatosa, WI. 53226

Project
Number

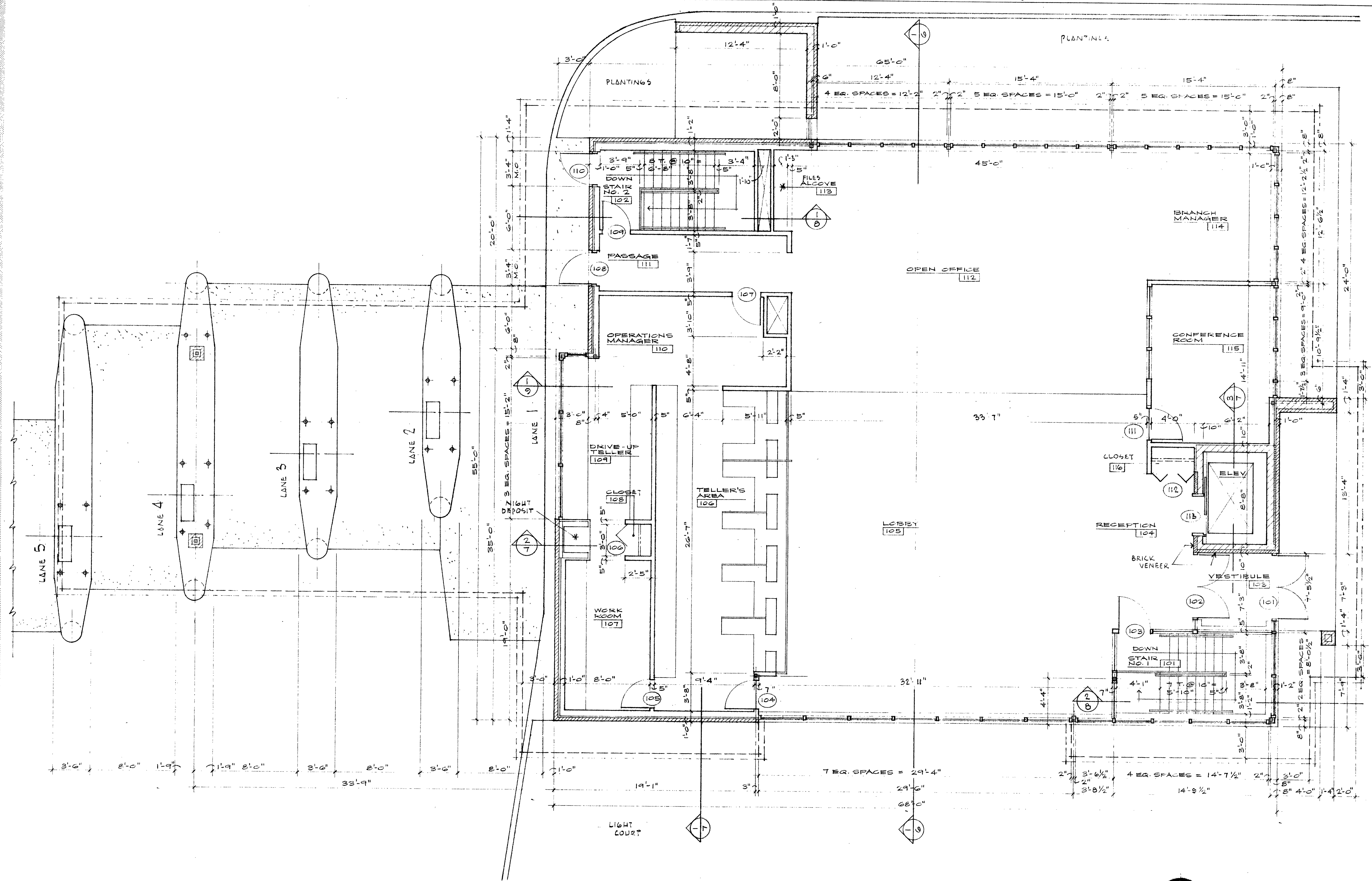
89214

1



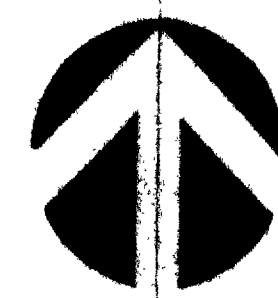
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FIRST FLOOR PLAN

SCALE 1/4" = 1'-0"



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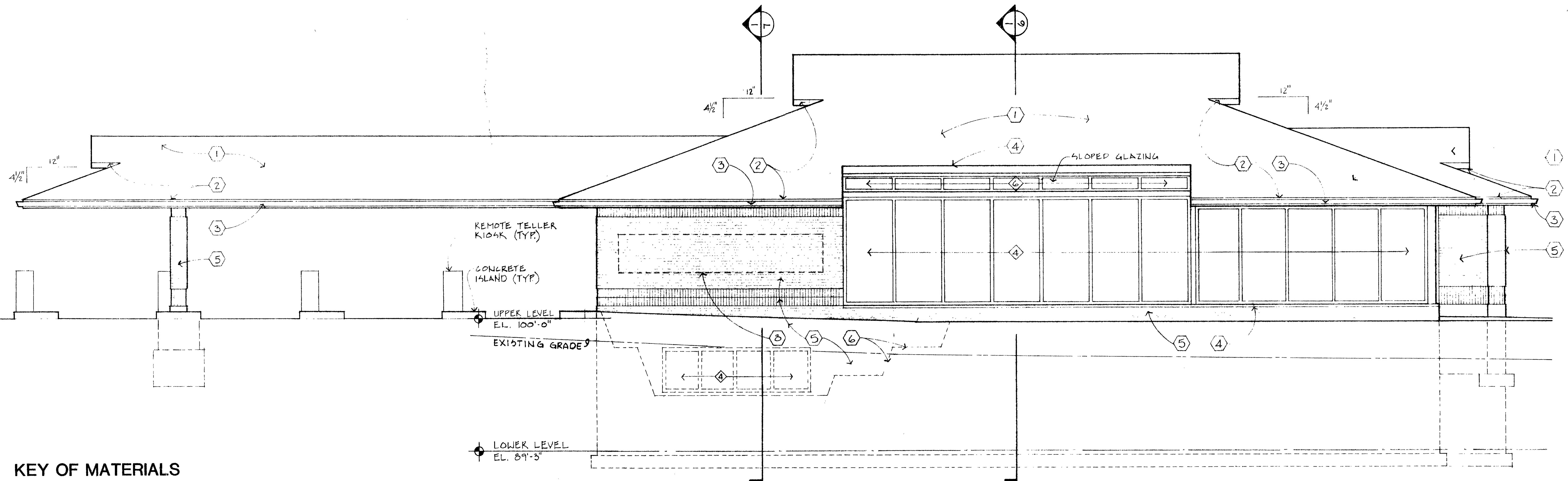
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Rev.	Date	Description	By	Appr.
1	11-5-89	GENERAL	BV	MH
2	10-30-89	III	III	III

Drawn
Date
Approved



KEY OF MATERIALS

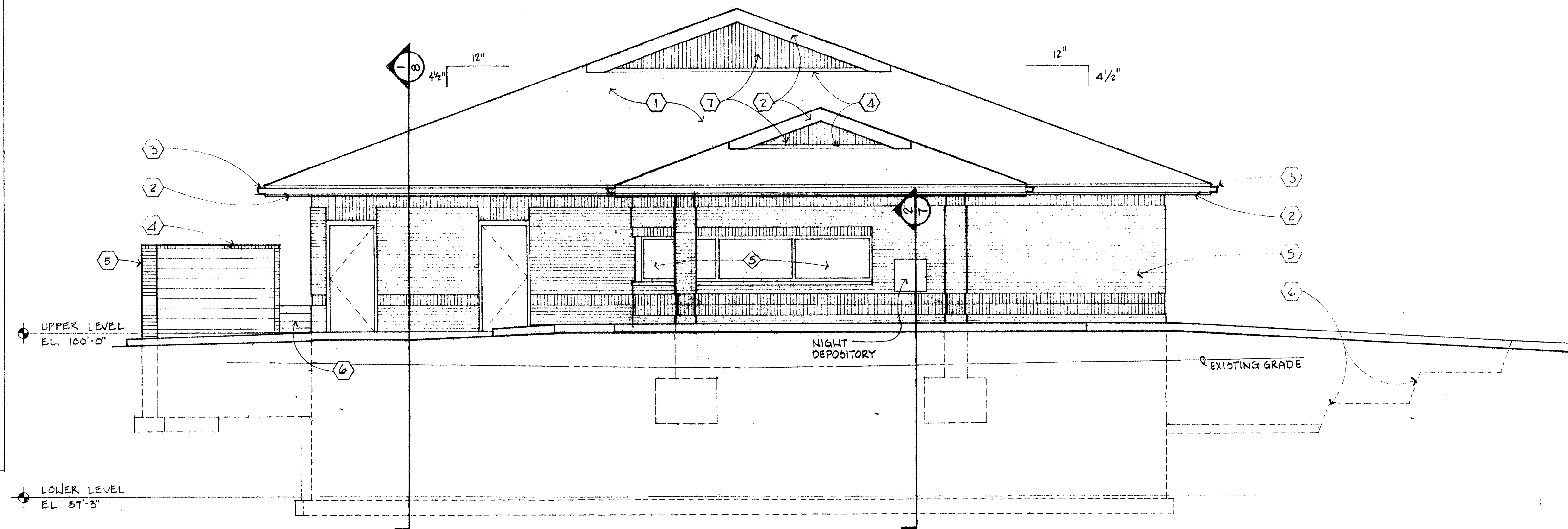
BUILDING MATERIALS

- ① CEDAR SHAKES (FIRE TREATED)
- ② PREFINISHED SHEET METAL FASCIA
- ③ PREFINISHED SHEET METAL GUTTER
- ④ PREFINISHED SHEET METAL FLASHING
- ⑤ FACE BRICK
- ⑥ RAILROAD TIE RETAINAGE BY LANDSCAPE CONTRACTOR
- ⑦ R.S. CEDAR TRIM (STAINED BY PAINTER)
- ⑧ SIGNAGE (N.I.C.)
- ⑨ BRICK LEDGE

GLAZING SCHEDULE

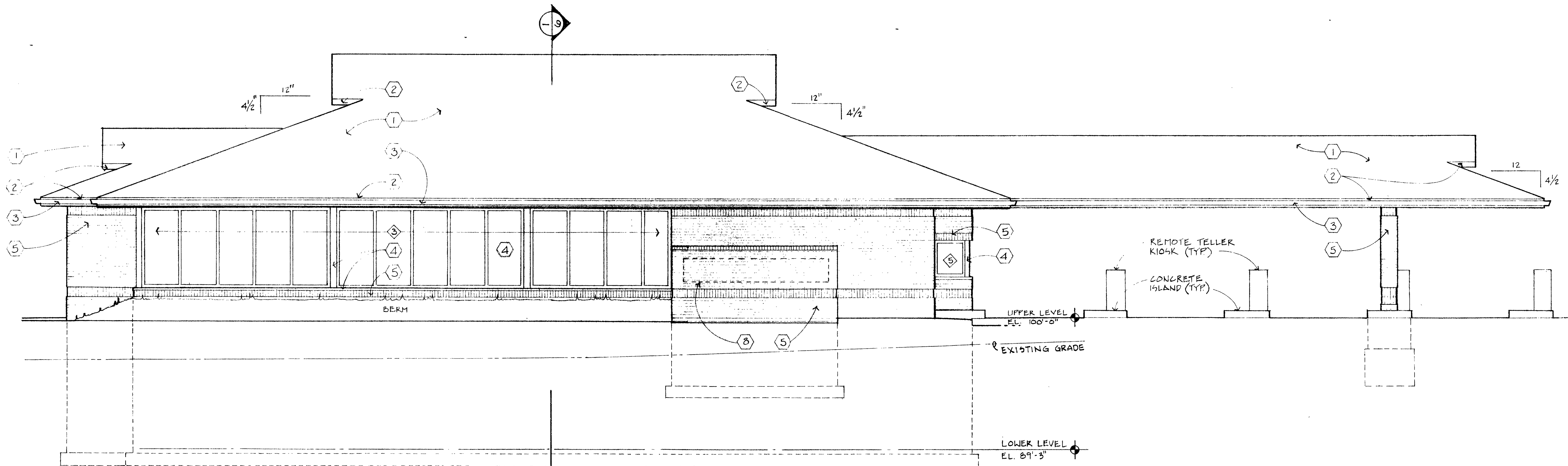
- ① BRONZE TINTED, TEMPERED, 1/4" THICK
- ② BRONZE TINTED, 1" INSULATING
- ③ BRONZE TINTED, 1" INSULATING, TEMPERED
- ④ BULLET RESISTANT GLASS
- ⑤ BRONZE TINTED, TEMPERED OUTER LITE, HEAT STRENGTHENED LAMINATED INNER LITE, 13/16" INSULATING

NOTE: ALL EXTERIOR GLAZING IS SET IN PREFINISHED METAL FRAMES

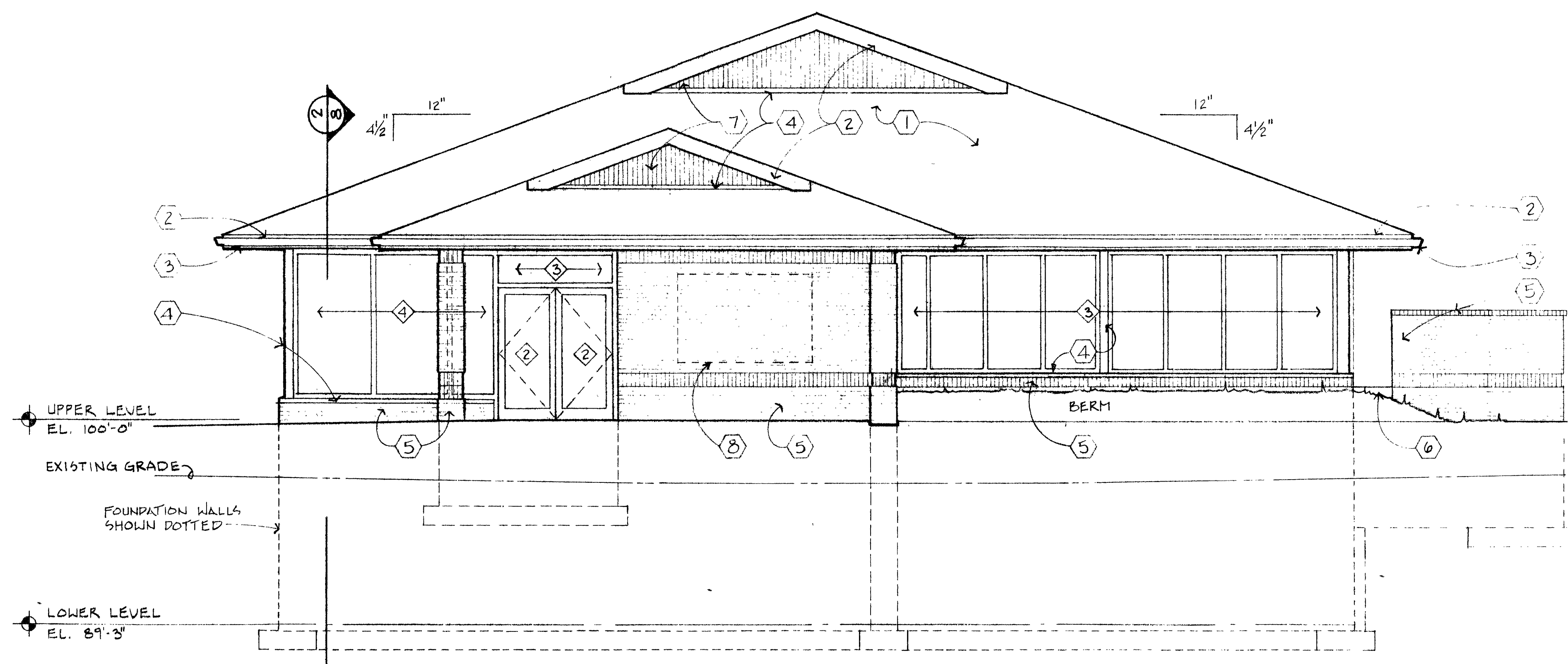


WEST ELEVATION

SCALE 1/4" = 1'-0"



NORTH ELEVATION
 SCALE 1/4" = 1'-0"
 0 2 4 10



EAST ELEVATION
 SCALE 1/4" = 1'-0"
 0 2 4 10

FOR KEY OF MATERIALS, SEE SHEET 4

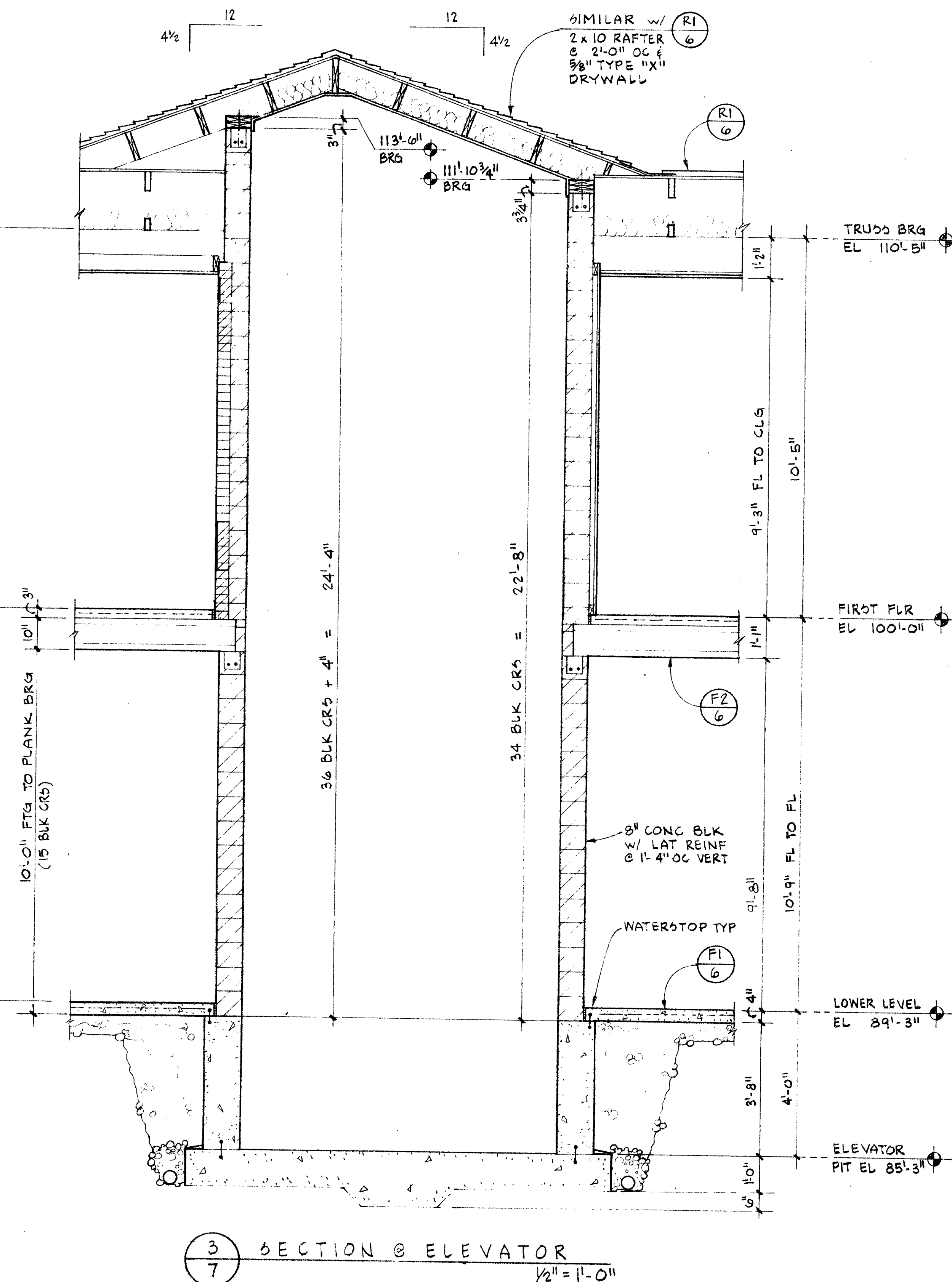
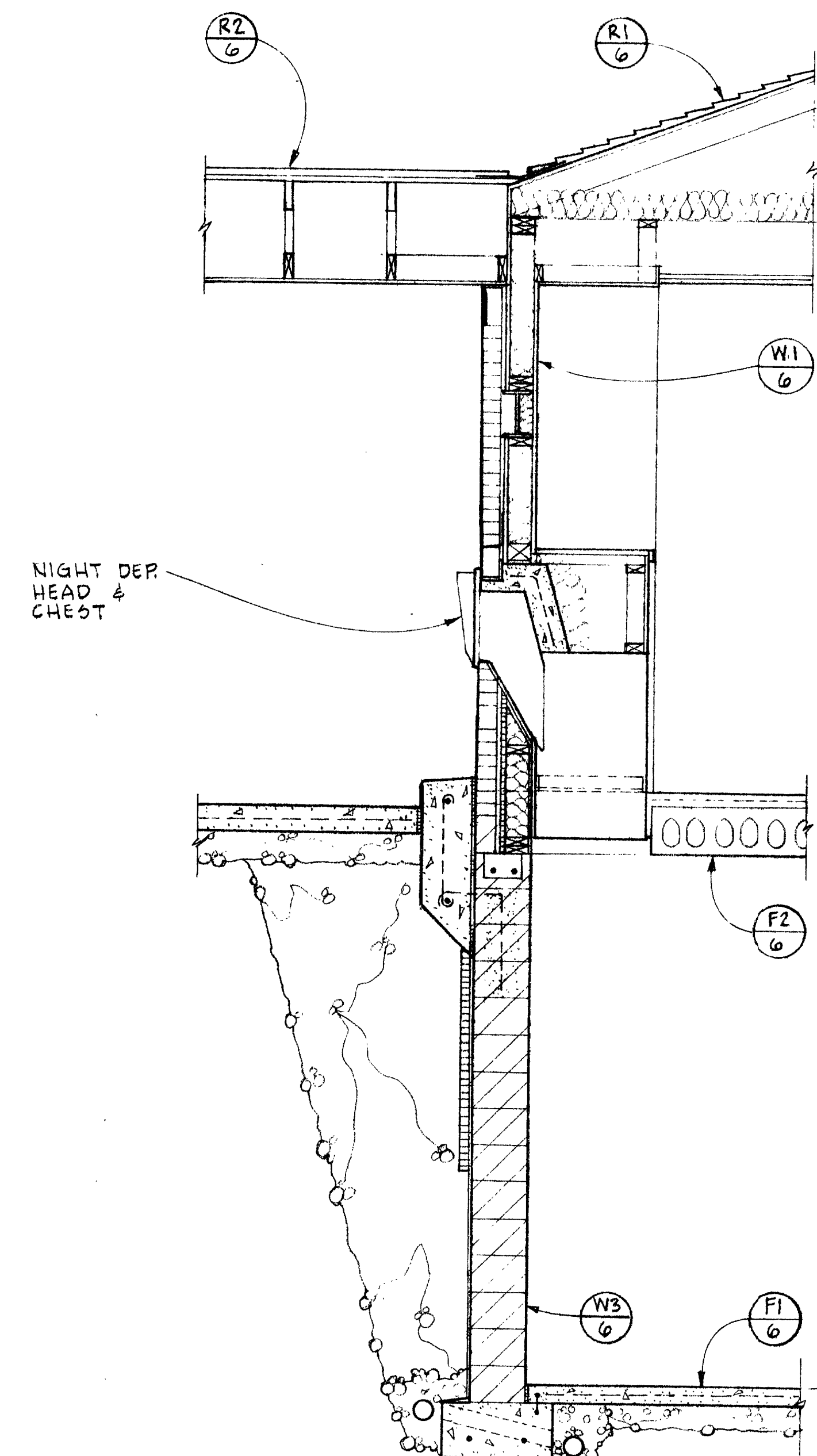
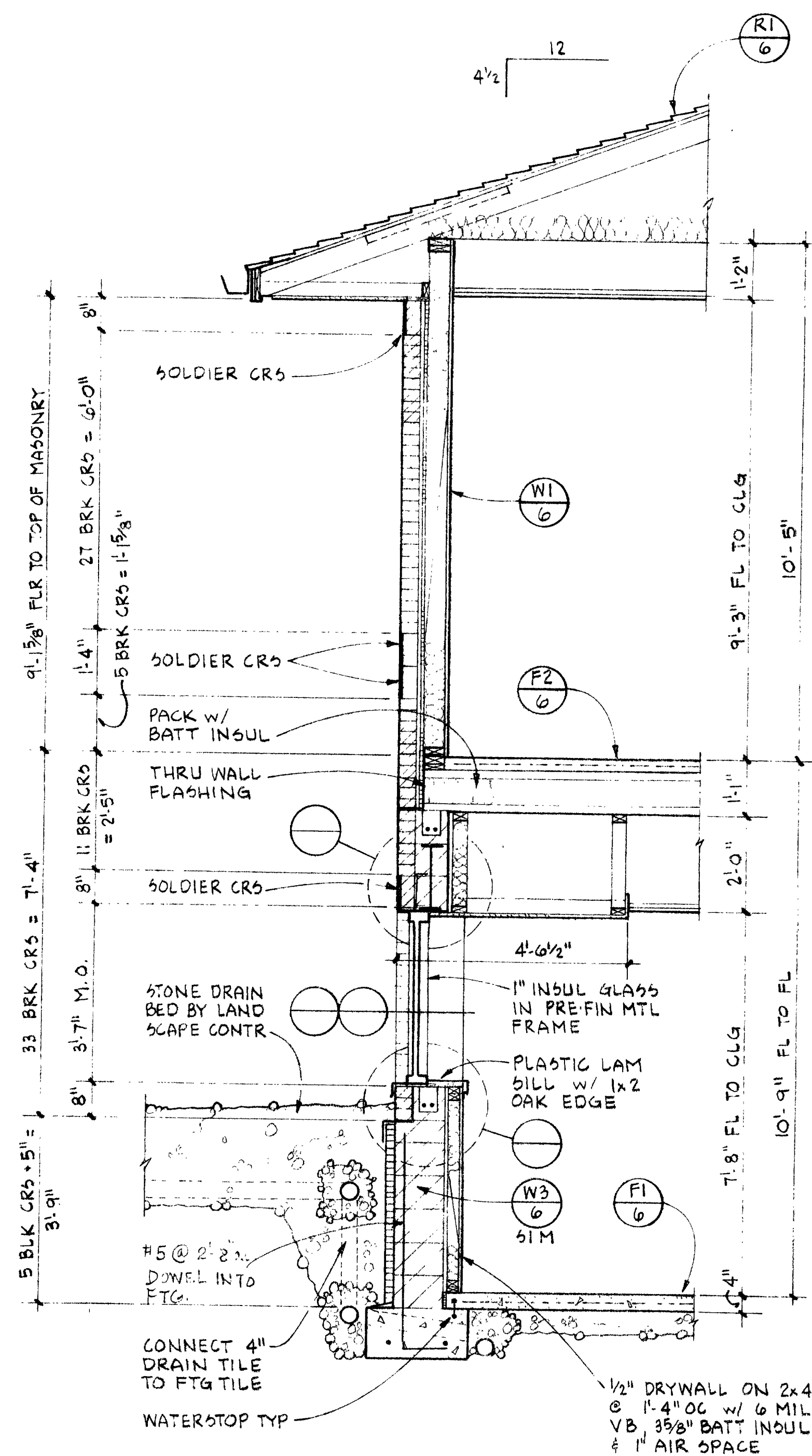
Rev.	Date	By	Description	Approved	Date
1	11.14.89	WB	GENERAL	WB/FJ	
2	10.30.89	WB	III	WB/FJ	

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 Brown Deer Road at
 64th Street Branch

engineers
architects
 (414) 453-4554

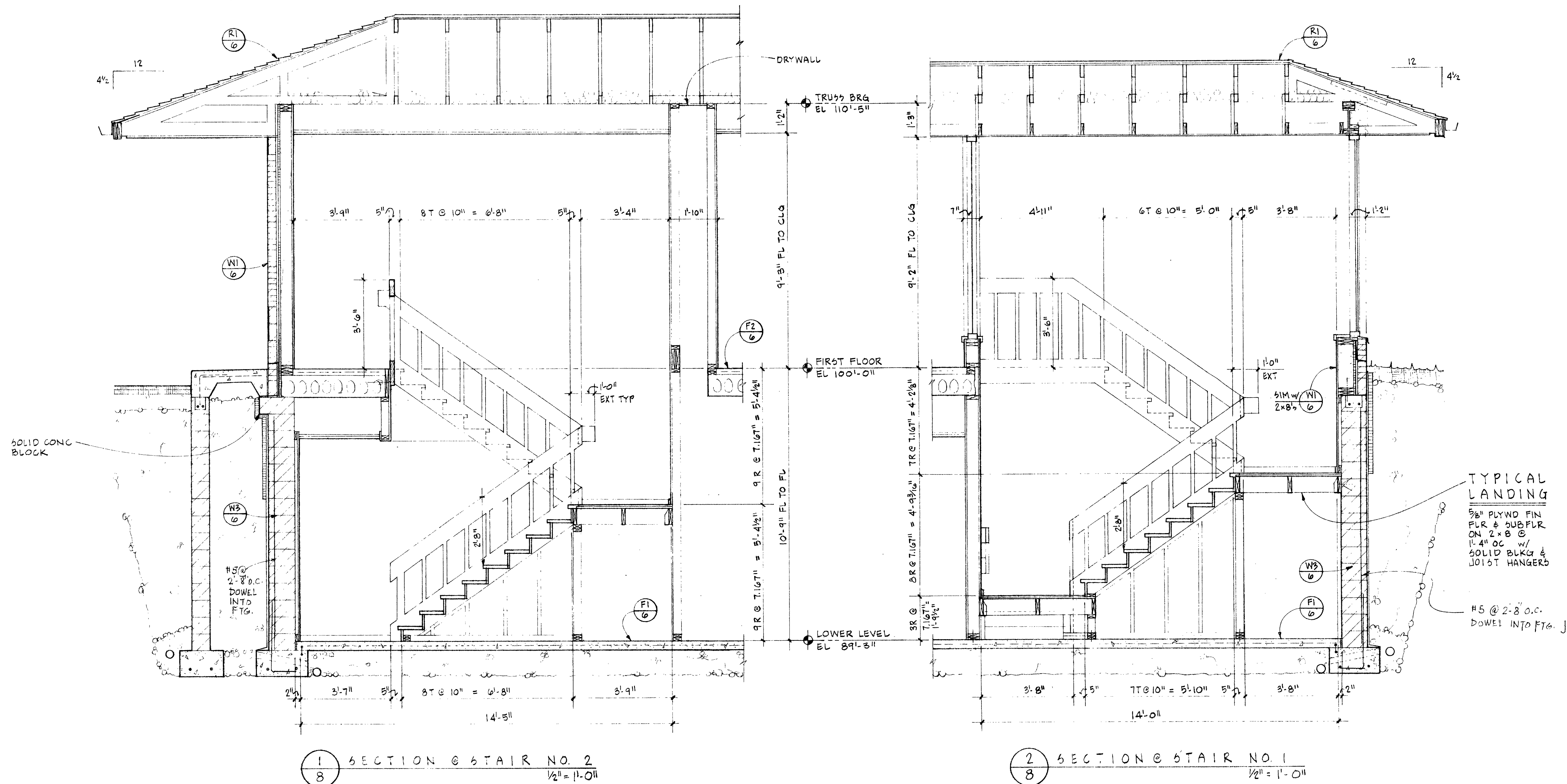
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Project Number
89214



TYPICAL STAIR CONSTRUCTION

- 1/4" FIR TREADS (EASE EDGES)
- 3/4" OAK RISERS
- 1 1/2" OAK STRINGERS
- 1 1/2" x 7/4" OAK HANDRAILS
(EASE TOP EDGES)
- 1 1/2" x 1 1/2" OAK BALUSTERS
- 10" O.C. MITER & DOWEL
INTO HANDRAIL & STRINGER



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1	11.6.88	111	Approved	Drawn

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COLUMN SCHEDULE

DESIGNATION	SIZE	BASE PLATE	ANCHOR BOLTS
C1	3x3x1/4"	3/4" x 10" x 0'-10"	(4) 3/4" x 1'-0" + 3" HK.
C2	4x4x1/4"	3/4" x 11" x 0'-11"	(4) 3/4" x 1'-0" + 3" HK.
C3	5x5x1/4"	3/4" x 12" x 1'-0"	(4) 3/4" x 1'-0" + 3" HK.
C4	6x6x1/4"	3/4" x 13" x 1'-1"	(4) 3/4" x 1'-0" + 3" HK.
C5	7x7x1/4"	1" x 14" x 1'-2"	(1) 1" x 3'-0" PLUS 3/4" x 6" W. OBL. NUTS @ END

GENERAL NOTES

- ALL MASONRY WALLS ARE 12" U.N.O.
- ALL STRIP FOOTINGS BENEATH MASONRY WALLS TO PROJECT 6" BEYOND EACH FACE OF WALL U.N.O.

99'-9" - TOP OF FOOTING ELEV.

6" CONC. SLAB W/ 1/4"
O 5" x 1/2" BOTHWAYS @
BOTTOM OVER 6" LAYER
OF HAY AND FILL (TYP)
AT ALL ENTRY DOORS

2x10 @ 16" MAX.

12" x 40" CONC. PIER
W/ (2) 5" x 3'-6" LONG
DOWELS INTO FOOTING.
TOP OF PIER ELEV. 99'-0"

FOOTING SCHEDULE

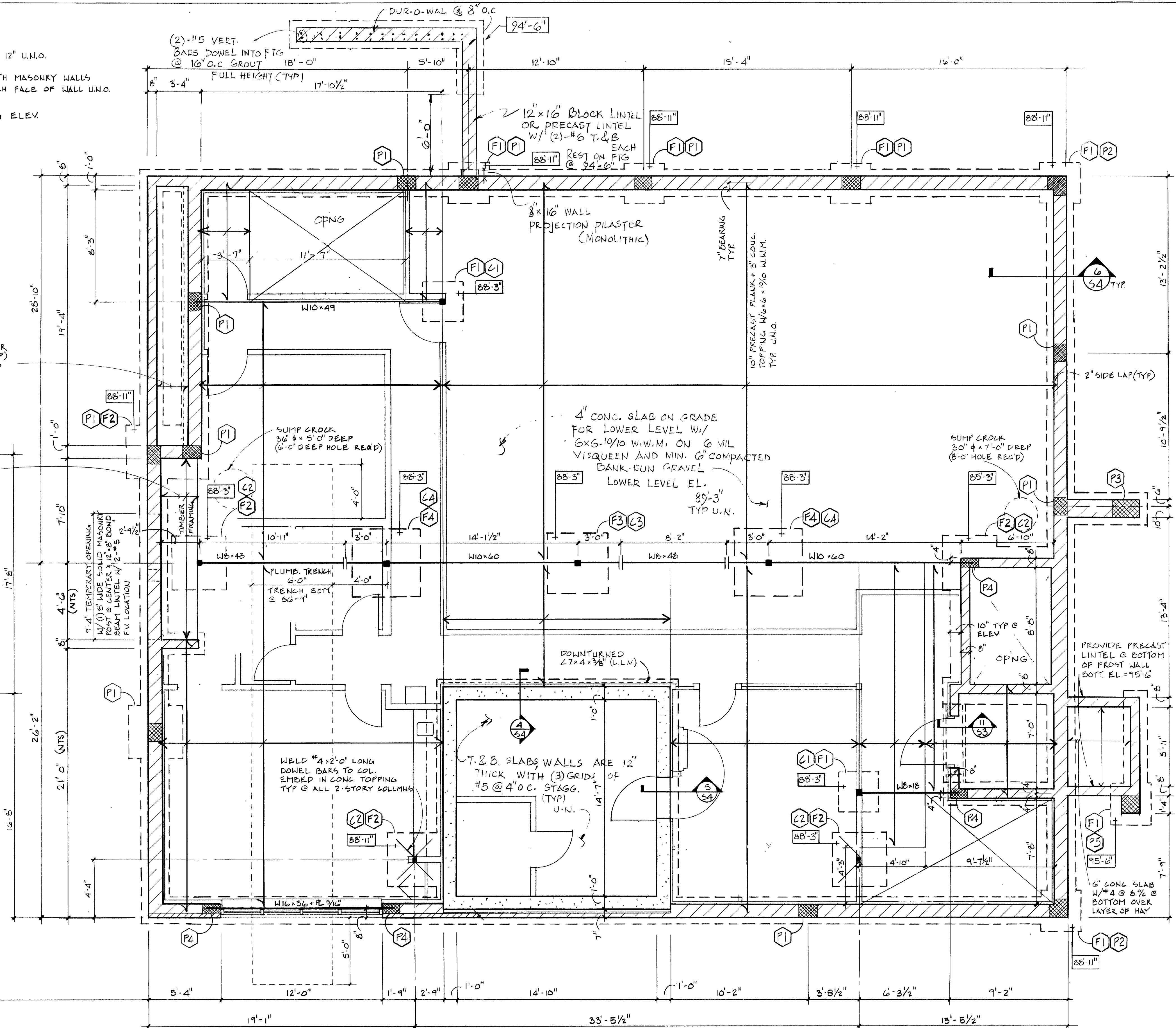
DESIGNATION	FOOTING SIZE	THICK-NESS	REINFORCING AT BOTTOM *
F1	3'-0" x 3'-0"	1'-0"	3-#5 x 2'-6" EA. WAY
F2	4'-0" x 4'-0"	1'-0"	4-#6 x 3'-6" EA. WAY
F3	4'-6" x 4'-6"	1'-2"	5-#6 x 4'-0" EA. WAY
F4	5'-0" x 5'-0"	1'-2"	6-#6 x 4'-6" EA. WAY
F5	4'-6" x 4'-6"	3'-0"	UNREINFORCED

* WHERE FOOTINGS ARE ON ROCK, NO REIN. RODS ARE REQ'D. REINFORCING INDICATED IS ONLY REQ'D WHERE FOOTINGS ARE ON SOIL OTHER THAN ROCK.

PILASTER SCHEDULE

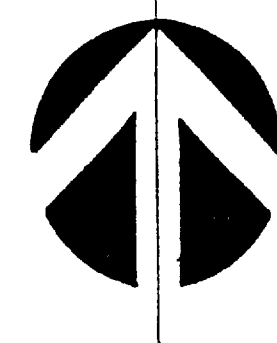
DESIGNATION	SIZE	VERT. REIN.
P1	12" x 16"	4-#5
P2	12" x 16" x 1/2"	5-#5
P3	16" x 24"	6-#5
P4	8" x 16"	4-#5
P5	16" x 16"	4-#5

PROVIDE DOWELS INTO FOOTINGS
TO MATCH VERTICAL BARS



LOWER LEVEL PLAN

SCALE 1/4" = 1'-0"



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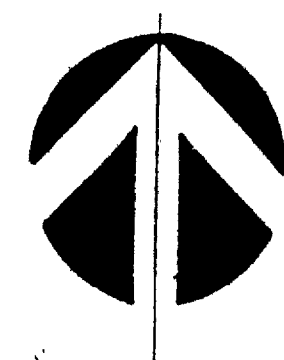
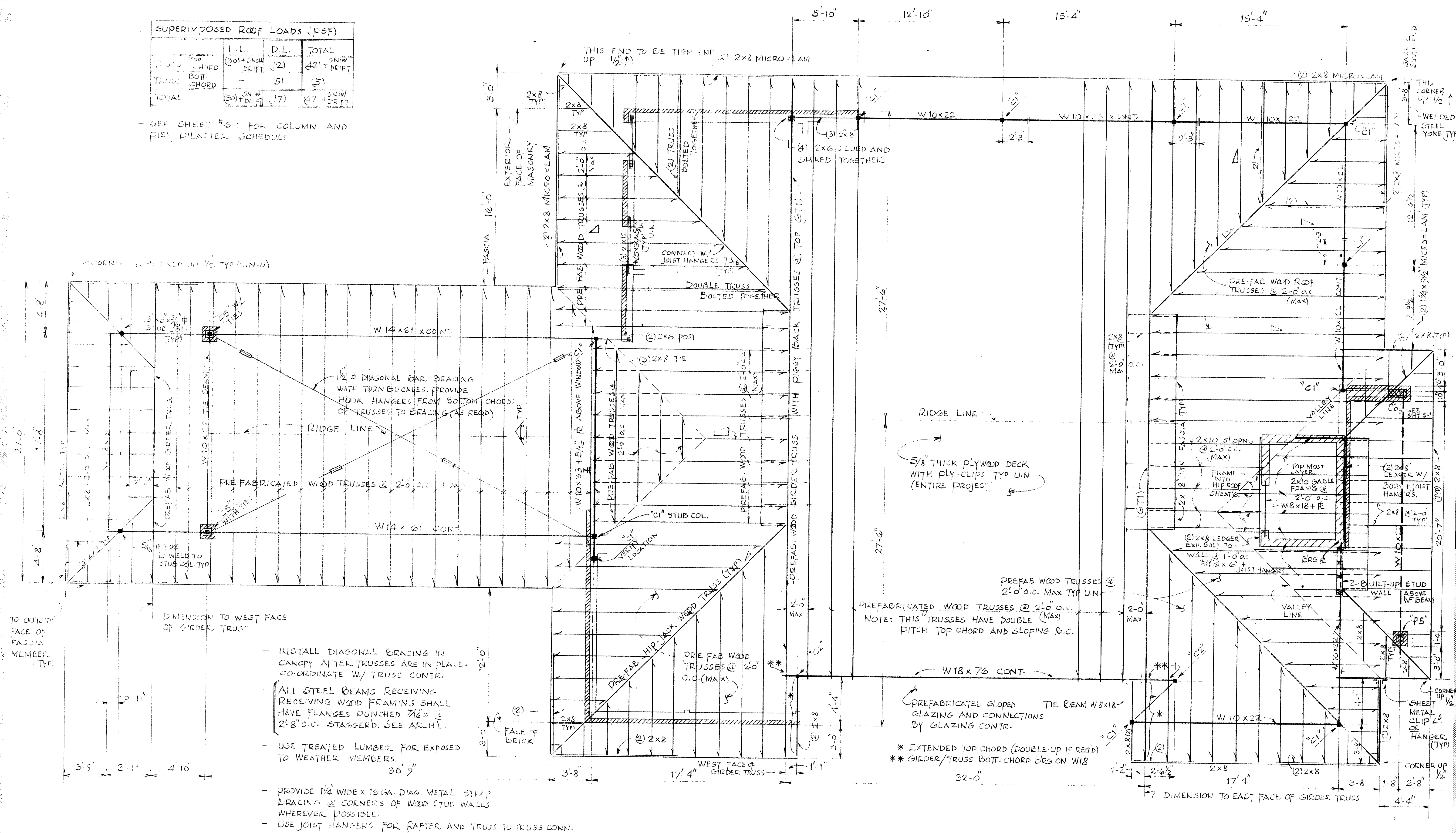
S1

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933 N. Mayfair Road Wauwatosa, WI 53226
architects engineers
(414) 453-4554

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Drawn
By
Description
Date
10-30-89
Approved
Date

SUPERIMPOSED ROOF LOADS (PSF)			
	L.L.	D.L.	TOTAL
TRUSS TOP CHORD	(30) + SNOW DRIFT	(2)	(42) + SNOW DRIFT
TRUSS BOT. CHORD	-	51	(5)
TOTAL	(30) SNOW DRIFT	(57)	(47) + SNOW DRIFT



ROOF FRAMING PLAN

SCALE: $1/4" = 1'-0"$

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S2

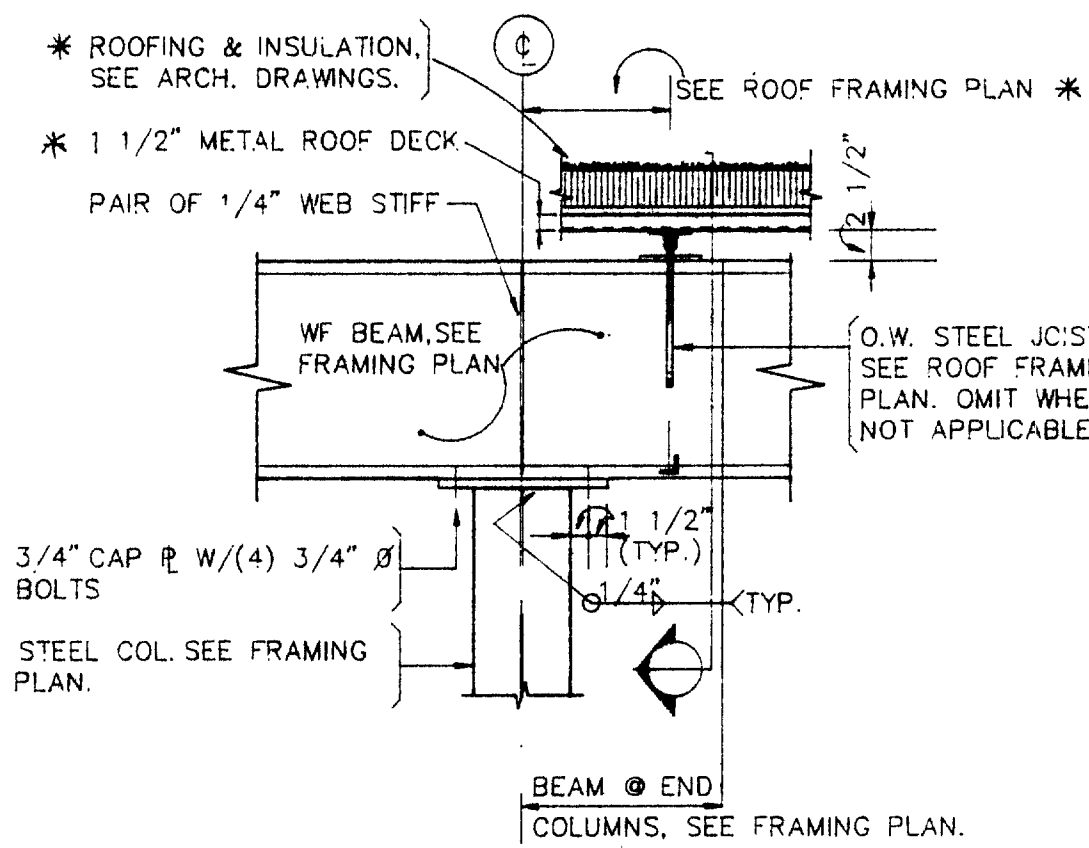


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architects **engineers**
(414) 453 - 4554

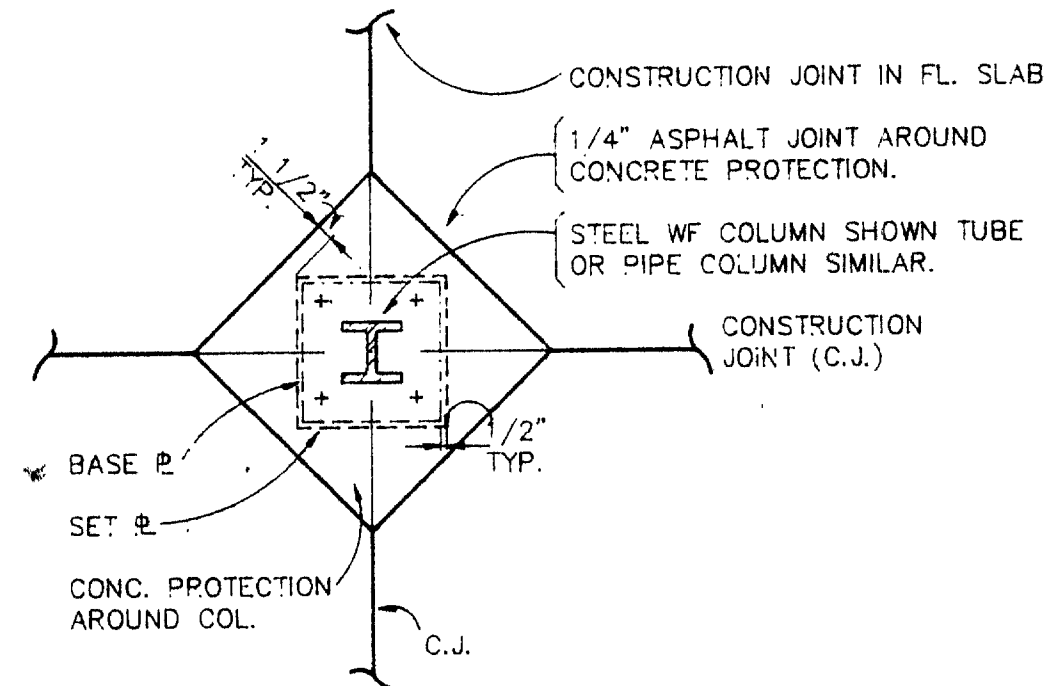
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Rev.	Date	Description	Bv
	11-6-89	Approved	RP GK Drawn



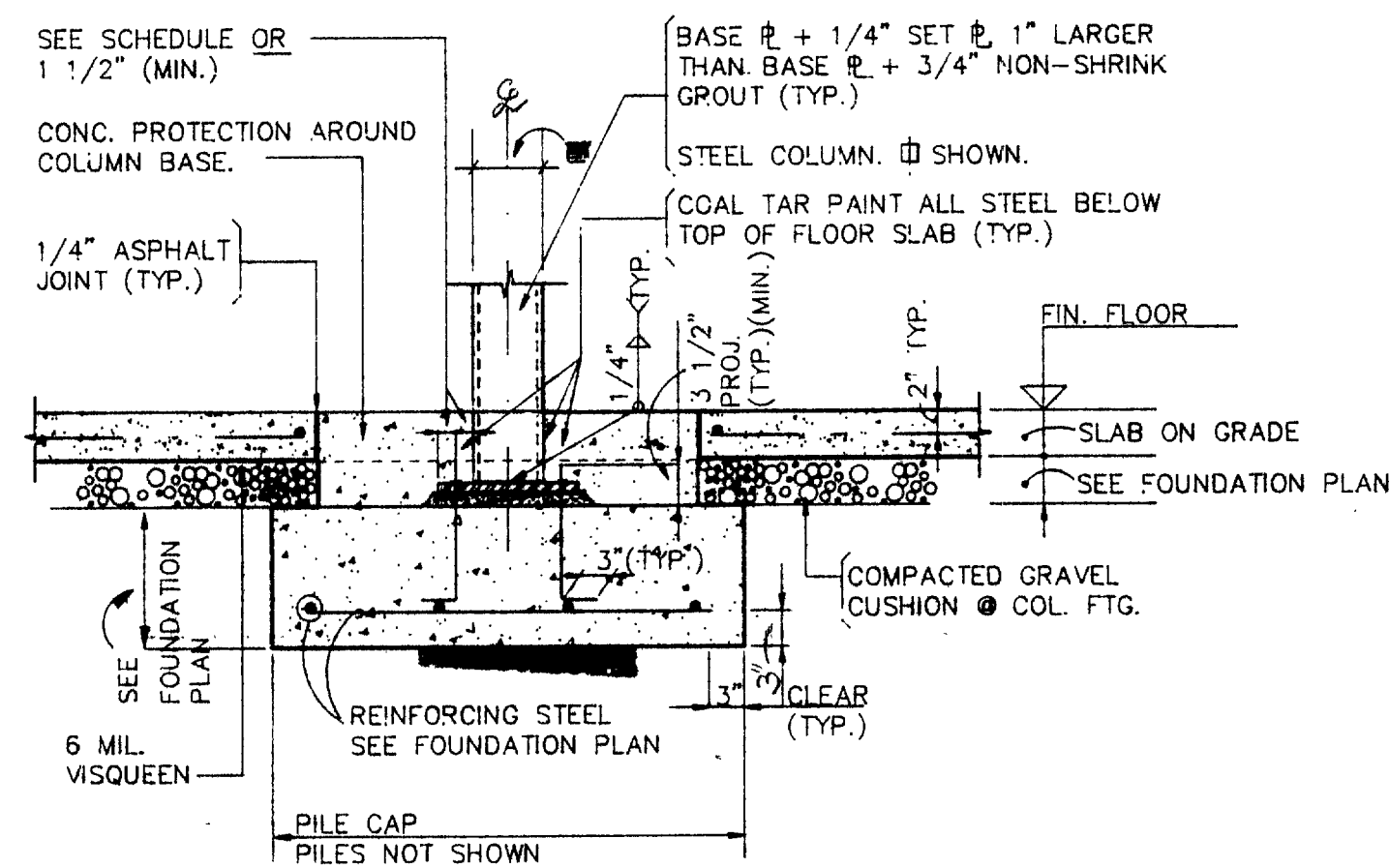
* OMIT FOR FLOOR FRAMING

1 BEAM-COLUMN CONNECTION DETAIL
5-3 SIM. DETAIL @ FLOOR FRAMING NO SCALE



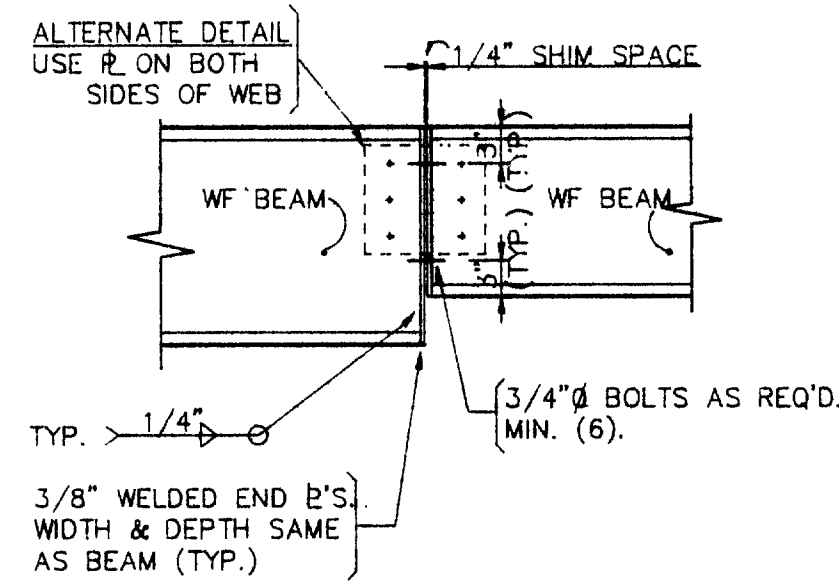
6 PLAN VIEW
5-3 NO SCALE

NOTE: SIMILAR DETAIL AT $\square$ OR $\circ$ COLUMNS.

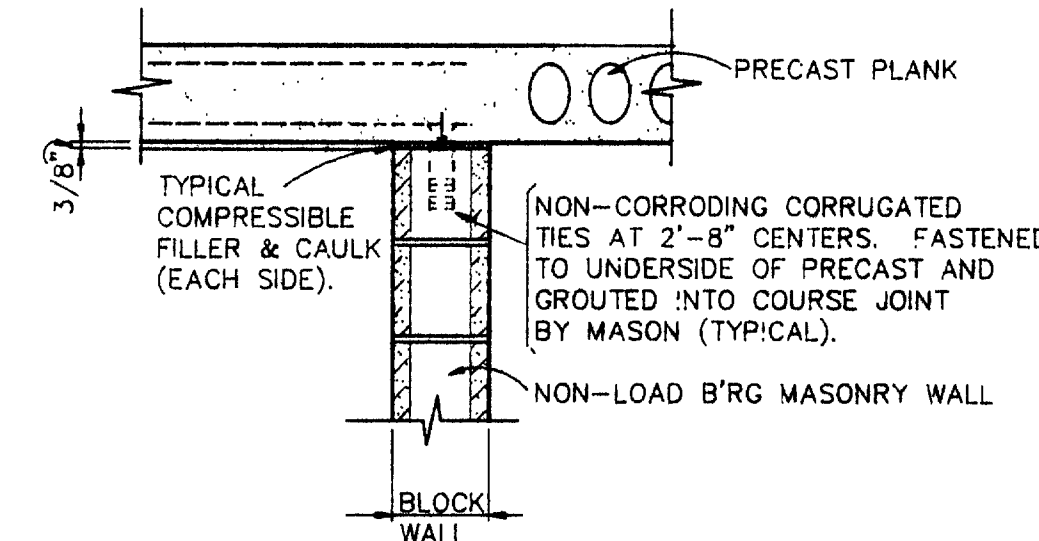


NOTE: SIMILAR DETAIL AT WF OR PIPE (ϕ) COLUMNS.

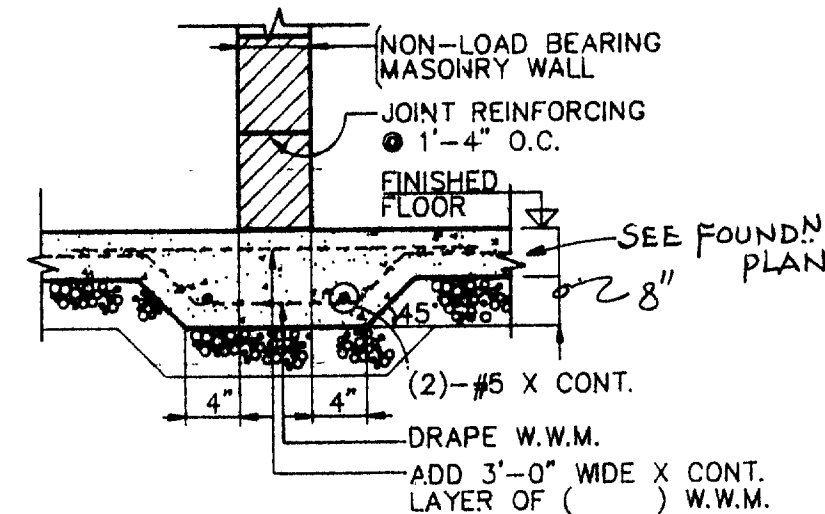
10 TYP. COLUMN FOOTING DETAIL
5-3 (INTERIOR COLUMN ONLY) NO SCALE



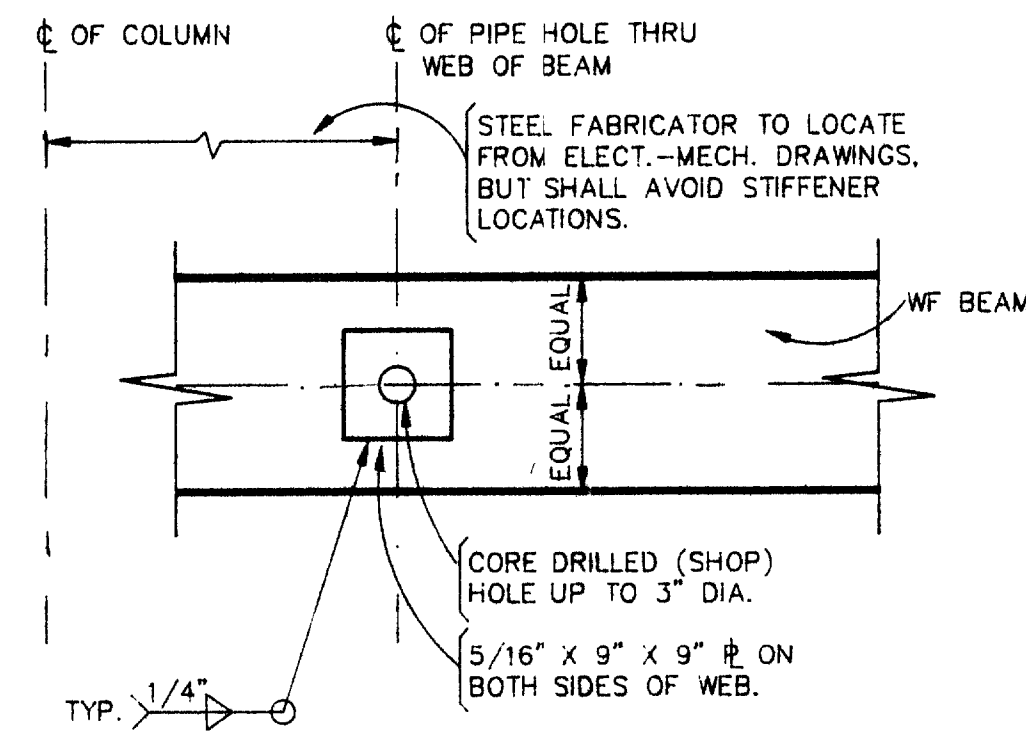
2 TYPICAL SHEAR SPLICE DET.
5-3 NO SCALE



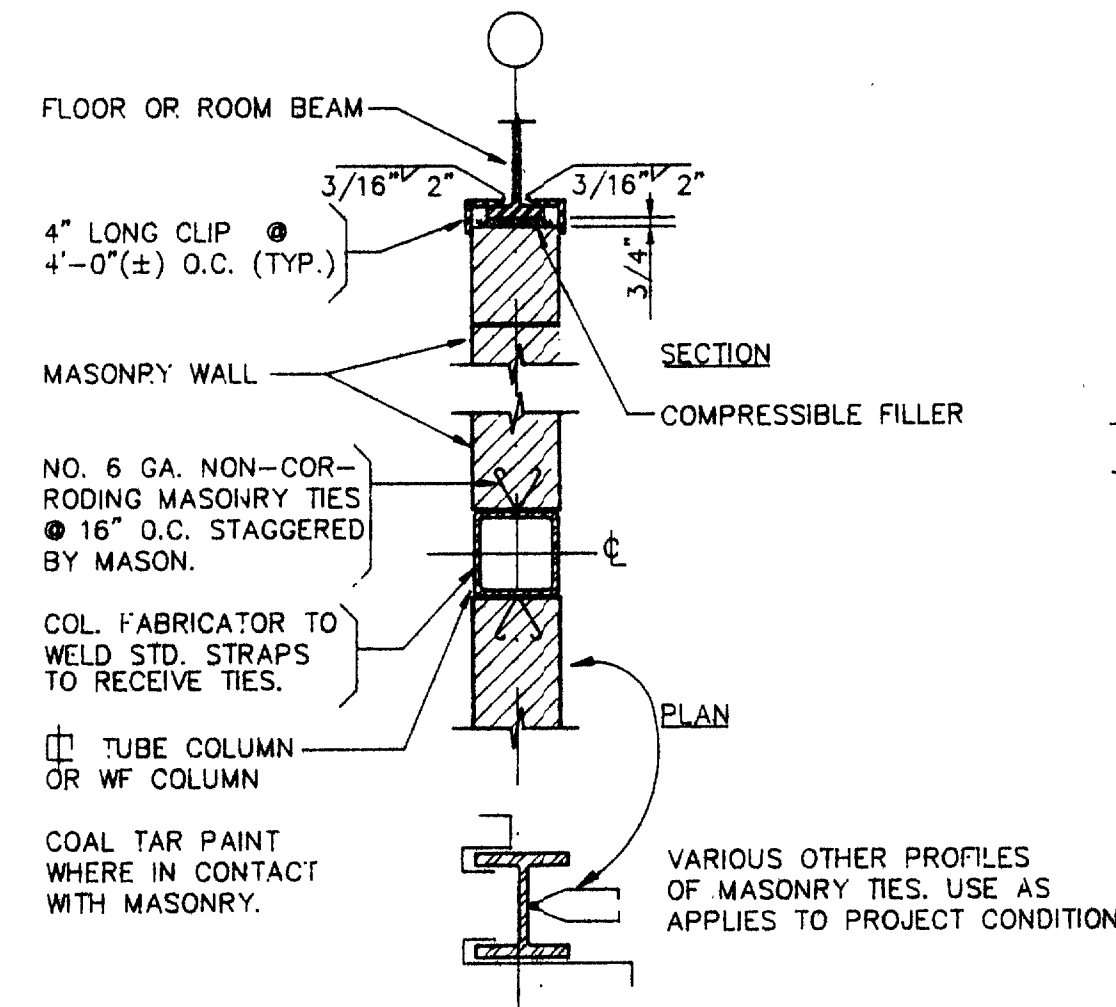
7 NON-LOAD BR'G MASONRY & P.C. CONC. DECK
5-3 DETAIL AT DOUBLE TT DECK SIM. NO SCALE



11 THICKENED SLAB @
5-3 INT. MASONRY WALL NO SCALE

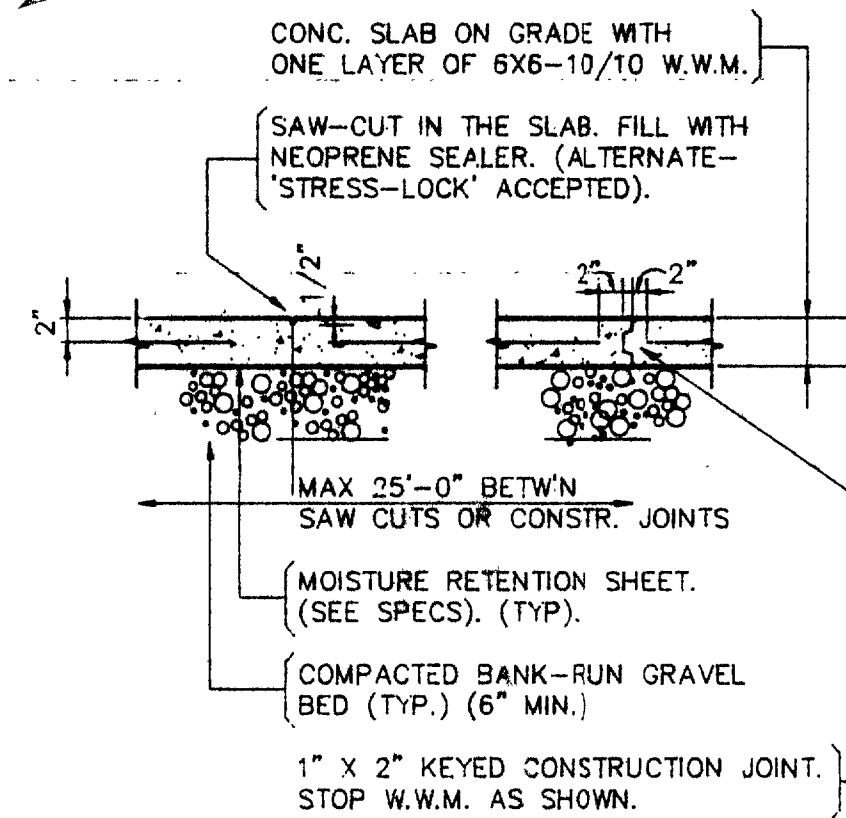


3 OPENING THRU WF BEAM
5-3 NO SCALE

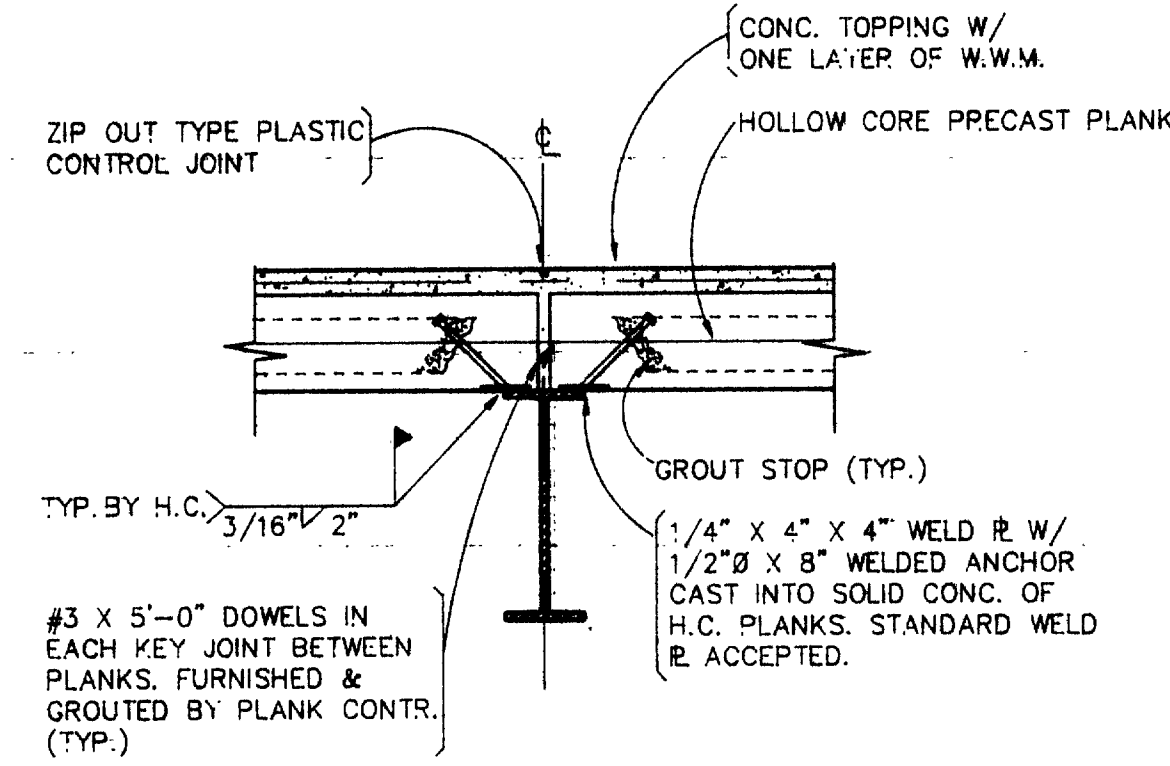


NOTE: SIMILAR TIE DETAIL WHEN COLUMN IS SURROUNDED BY MASONRY ON (3) OR (4) SIDES.

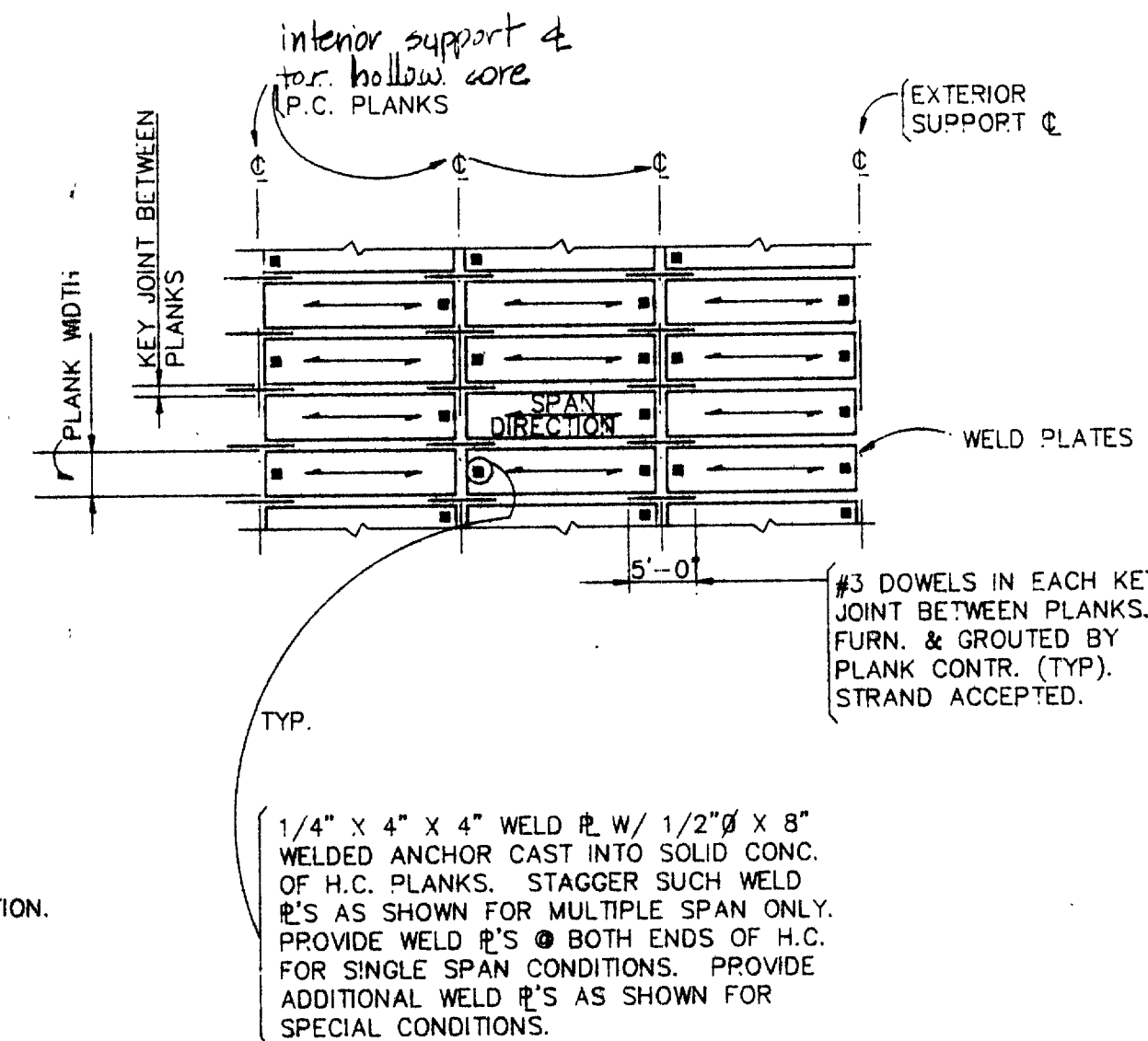
8 'WF' BEAM, COLUMN & MASONRY
5-3 NO SCALE



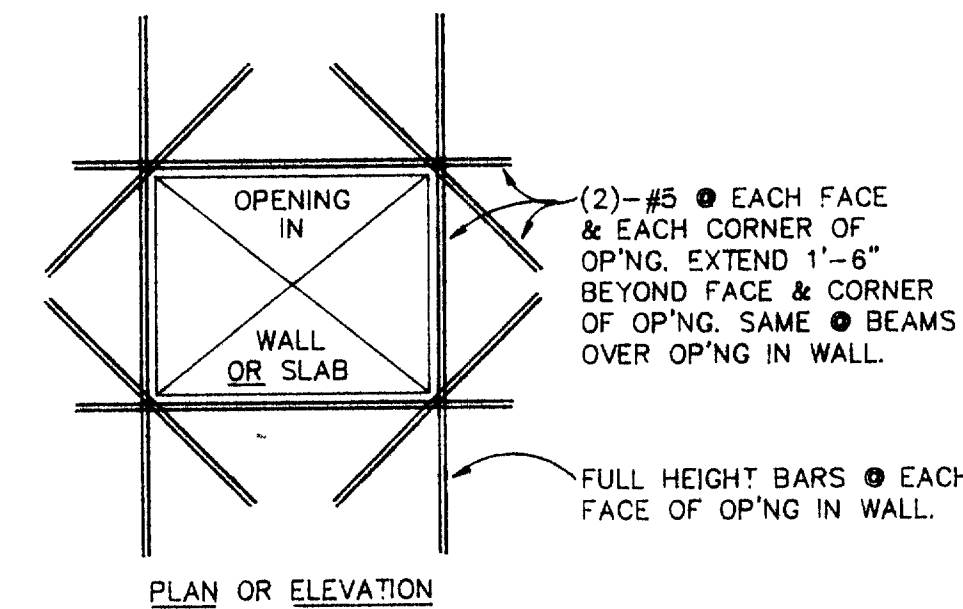
12 TYP. SAW-CUT & CONSTR. JT. IN
5-3 FLOOR SLAB NO SCALE



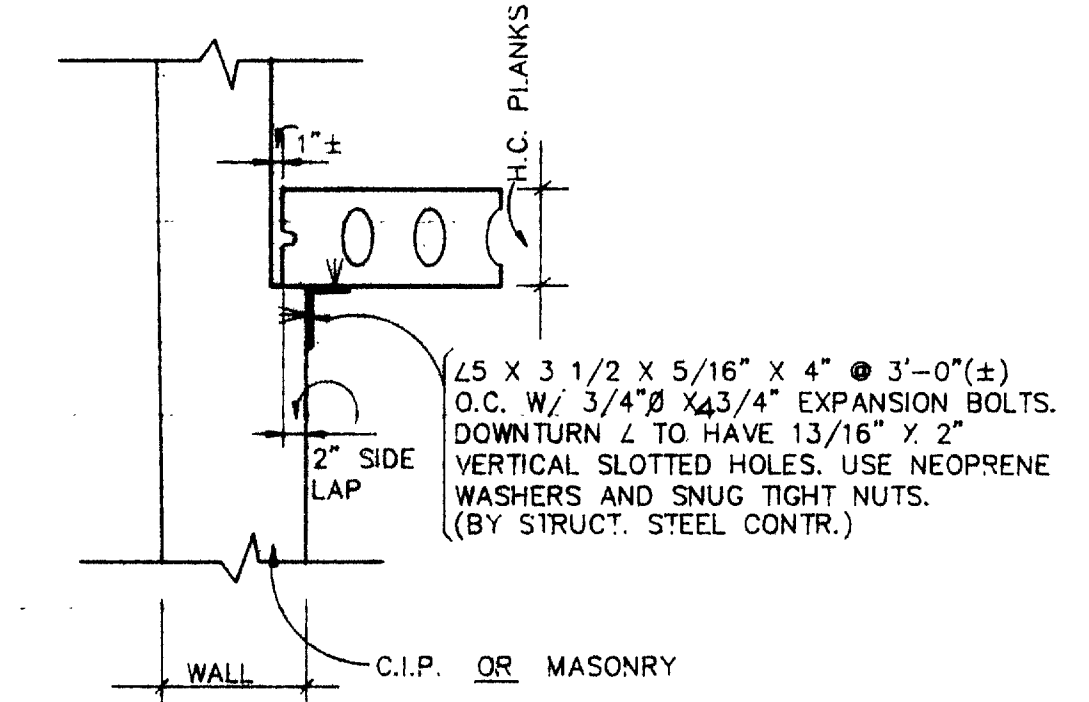
4 PRECAST PLANK-WF BEAM CONNECTION
5-3 NO SCALE



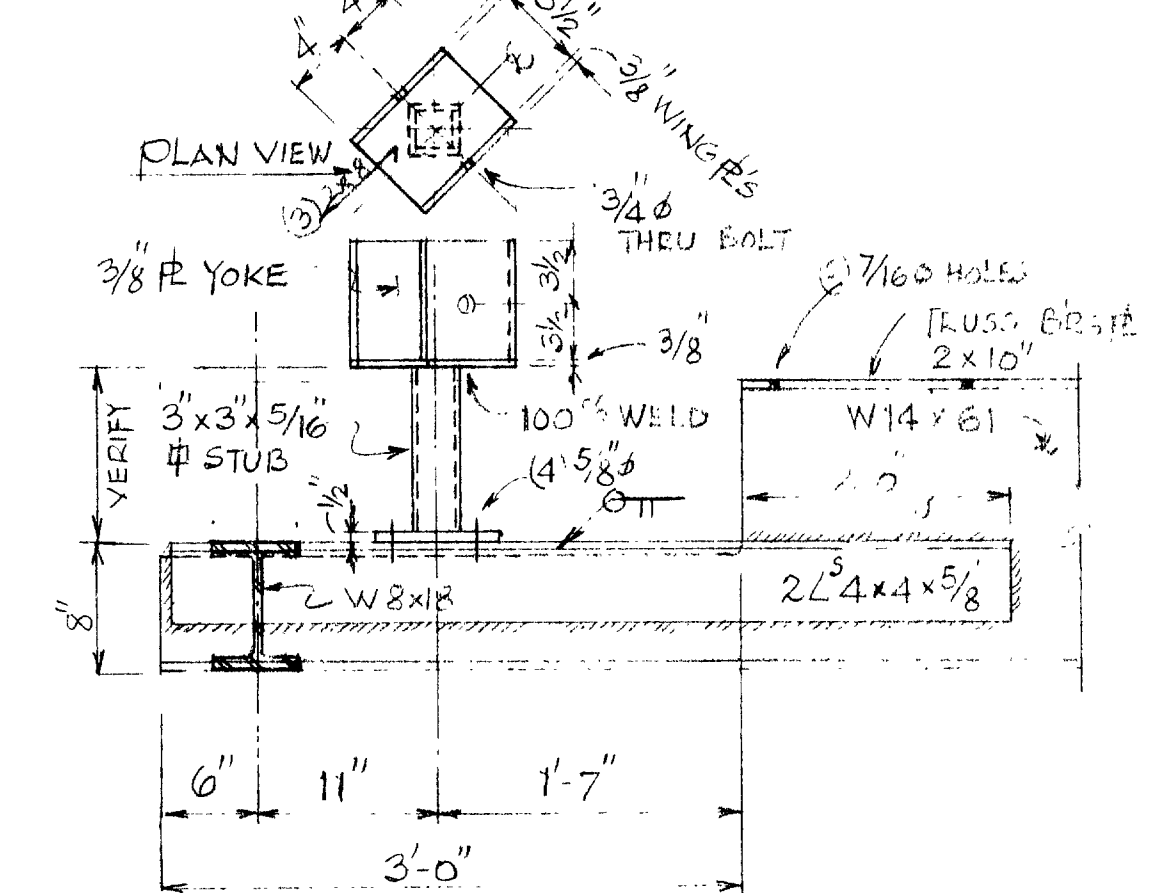
9 REFLECTED CEILING PLAN
5-3 AT PRECAST PLANKS NO SCALE



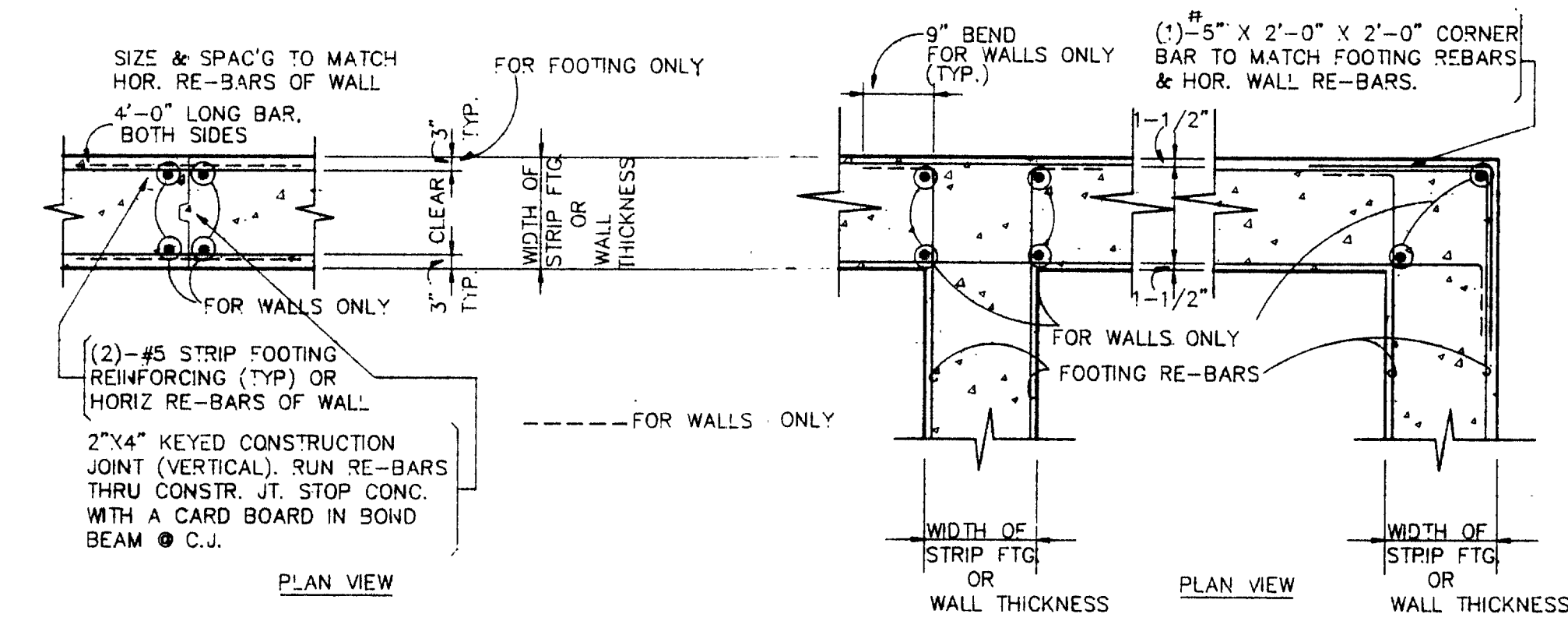
13 TYP. ADDITIONAL RE-BARS @
5-3 OP'NGS IN WALLS NO SCALE



5 H.C. PLANK SIDE LAP
5-3 NO SCALE



14 CANOPY BEAM AT WEST END
5-3 1\"/>

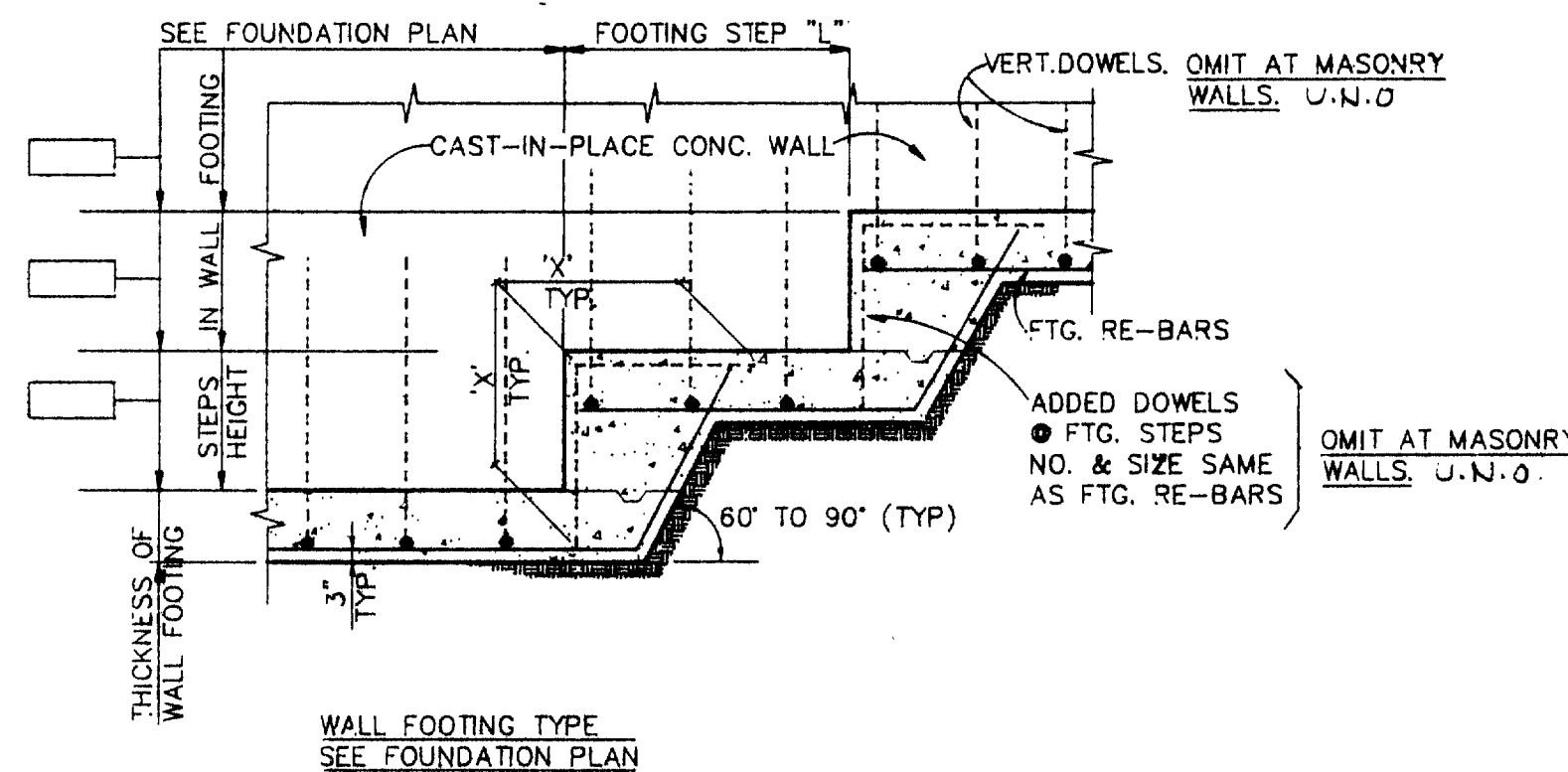


NOTE: PROVIDE "PVC" WATER STOP @ EXTERIOR FACE WHEN NEEDED

NOTE: SIMILAR DETAILS AT TRENCH FOOTINGS AND GRADE BEAMS

① TYP. CONSTR. JOINT IN WALL AND WALL FTG. NO SCALE

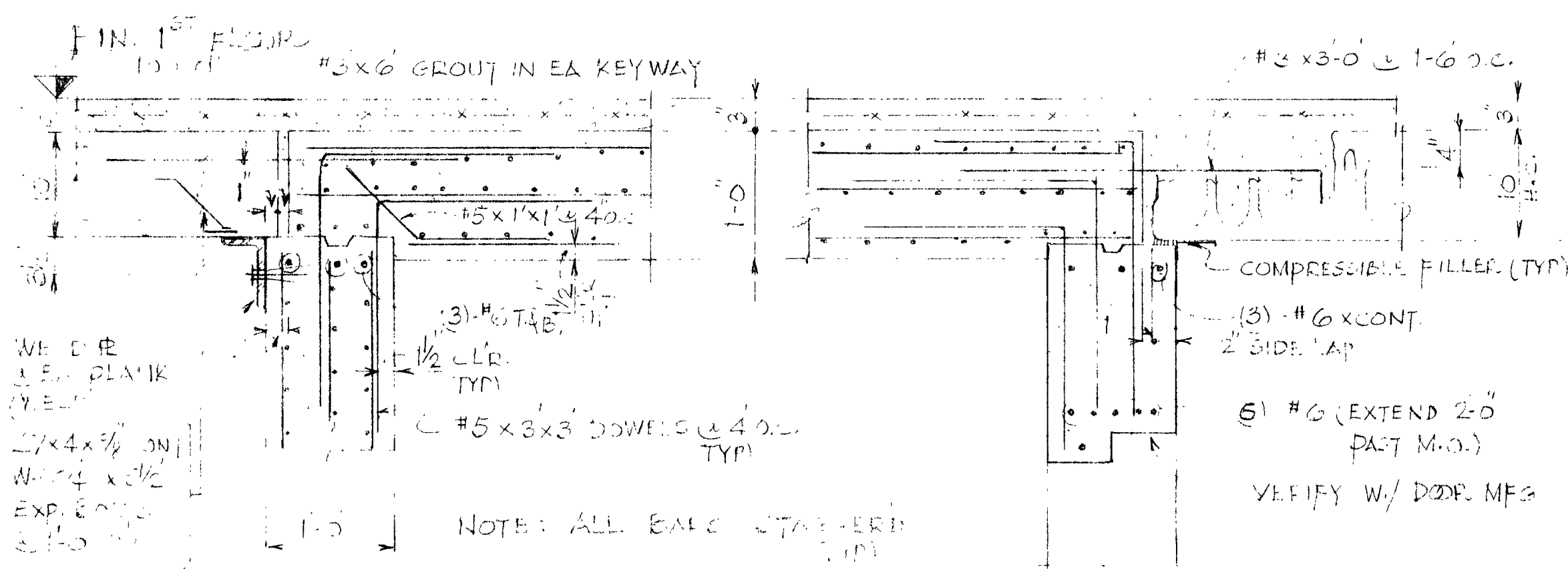
② TYP. CORNER REINF. IN WALL AND WALL FTG. NO SCALE



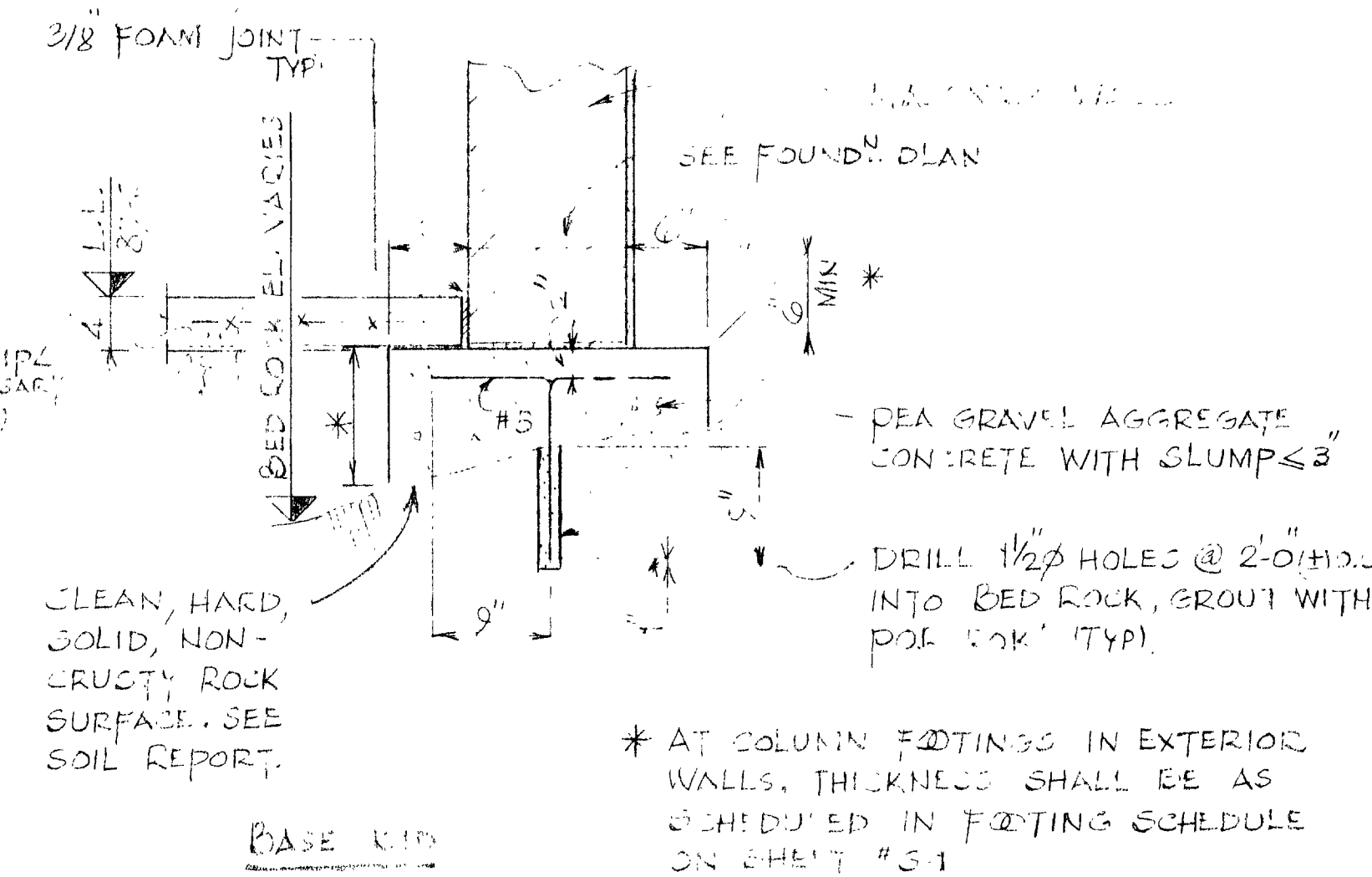
NOTE: SIMILAR DETAILS AT TRENCH FOOTINGS.

OMIT VERTICAL DOWELS @ MASONRY WALLS U.N.O.

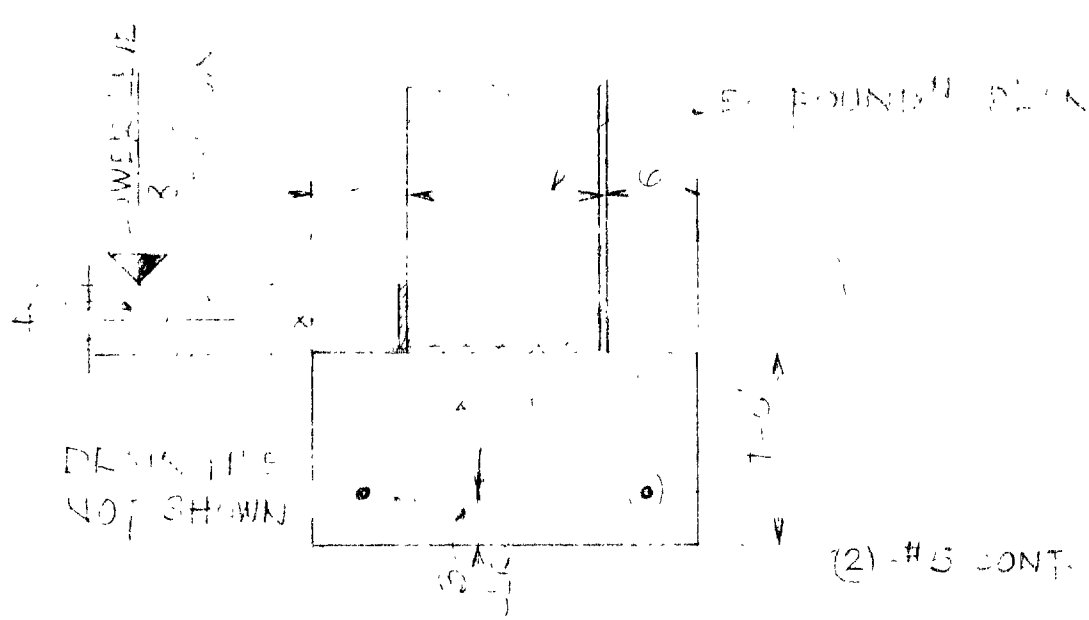
③ DETAIL AT STEPS IN WALL FTG. NO SCALE



④ EXTERIOR WALL FOOTING ON ROCK (6) INTERIOR FOOTING ON ROCK SIMILAR SCALE: 1"=1'-0" S-4



EXTERIOR WALL FOOTING ON ROCK (6) INTERIOR FOOTING ON ROCK SIMILAR SCALE: 1"=1'-0" S-4



NOTE: CONTACT TO SURVEY UNIT FOR PRELIMINARY OR REVISIONS FOR THE CONSTRUCTION

⑦ EXTERIOR WALL FOOTING IN SOIL SCALE: 1"=1'-0"

STRUCTURAL NOTES AND SPECIFICATIONS (AS APPLIES) FOR THE ENTIRE PROJECT:

- Strength of Materials:
 - Concrete $f_c' = 4,000$ psi (air-entrained for exposed to weather concrete)
 - Re-bars $f_s = 24,000$ psi (Grade 60)
 - Masonry heavy aggregate concrete block below grade
 - Mortar Type 'S'
 - Structural and miscellaneous steel A36 U.N.O. (painted)*; tube and pipe sections 46 Ksl (painted)*
 - Connection bolts A325
 - Anchor bolts A307
 - Steel encased in or in contact with masonry shall be painted with bituminous paint. Steel exposed to weather shall be prime painted with rust inhibitive paint.
- Minimum soil bearing capacity: (soil bearing log available with Owner AND IS MADE PART OF CONSTRUCTION DOCUMENTS) 3,500 psf (Rock)
- Follow latest ACI, CRSI, and all applicable loads.
- Follow latest AISC Code for design, fabrication and erection of structural and miscellaneous steel.
- New concrete footings shall rest at elevation shown on foundation plan. 95'-4" represents top of column footing or wall footing.
- Bottom of the new wall footing shall be the same as the bottom of the existing footing at juncture of new and existing. Use road-slab type anchors (smooth $3/4" \times 2'-0"$) at connection. At expansion joints, smooth bar shall be encased in closed end, grease filled sheet metal tubing. Underpin existing foundations in segments.
- If for some unforeseen reason it becomes necessary to drop a column footing, Concrete Contractor shall provide a suitable concrete pier with minimum reinforcing per ACI.
- Concrete Contractor shall be responsible for coordinating and providing concrete platforms/pedestals for electricals and mechanicals as and where required. Provide $1/2"$ Isolator joint between pads and floor slab.
- At no place shall the thickness of the floor slab be less than specified. Concrete Contractor to locate and provide recesses in floor slab for plumbing, electricals and mechanicals.
- See architectural drawings for additional dimensions and information.
- Abbreviate U.S(N).O. on drawings means "UNLESS SPECIFIED (NOTED) OTHERWISE"
- All steel columns shall, unless specified otherwise, have $3/4"$ cap and base plate with $3/4"$ bolts.
- Unless specified otherwise, provide $(5/8" \times 80.00)$ square inch area (minimum) bearing plate where beams/intels are masonry bearing. Bearing plate shape shall be such as to accommodate wall and beam. Weld beam/intel to plate. Provide two $5/8" \times 6"$ Nelson studs to plate and grout solid into wall.
- Provide pair of full height, full width $1/4"$ web stiff, in beams continuous over column at column.
- Cantilevered beams for framing shall be fabricated so that natural camber raises cantilevered end.
- Do not cut or burn holes thru new and/or existing steel members without written permission from the Architect.
- HVAC Contractor(s) shall be responsible for coordination of locations and sizes of openings required for their phase of work. If openings and/or support system for the equipment is different than shown on the drawings Mechanical Contractor shall notify the Architect and the Structural and Miscellaneous Steel Contractor prior to the fabrication of steel framing. Provide necessary temporary and permanent bracing/bridging for support of trusses and framing.
- Contractor shall adequately guy and brace all structural components to maintain safety and alignment during all phases of construction. Such guying and bracing shall remain in place until structure has reached adequate strength and is permanently braced.

NOTES FOR HOLLOW CORE PLANK CONTRACTOR

- Notes on this drawings and/or other drawings for the project shall hold true for the entire project where applicable.
- This Contractor shall be responsible for locations, verification of sizes and shop cutting of openings in the h.c. precast prestressed planks. This Contractor shall also design, fabricate and erect (or cast) headers for such openings as well as planks around columns and/or pilasters. In case of missing openings on framing plans, but shown or required on mechanical drawings, mechanical drawings shall govern.
- Submit adequate number of copies of shop drawing and calculations to the Architect and the state and/or local plan approving authorities together with the proper application forms and fees for approval of such drawings.
- Loading as well as fire ratings for different levels shall be in compliance with State, local and/or other applicable code requirements for the design of hollow core planks and other precast concrete members. In case of missing load and fire rating data, a verification with the Architect for such requirements is recommended.
- Where a non-load bearing masonry wall is running parallel to and resting on top of h.c. planks, this Contractor shall provide "tie-downs" to adjoining planks.

NOTES FOR PREFABRICATED WOOD TRUSSES

- All prefabricated wood trusses shall be designed for the applicable loads and span parameters by the truss contractor. DILHR requires shop drawings with engineering data for such trusses with a stamp and signature of a registered professional engineer from the State of Wisconsin. The following data for wood trusses must be followed for the design:
 - Wood Truss Profile Data
 - See framing plans for superimposed design loads.
 - Truss spacing = (see framing plans)
 - Live load deflection: $L/360$ or better.
- Provide necessary temporary and permanent bracing/bridging for support of trusses.
- Contractor shall adequately guy and brace all structural components to maintain safety and alignment during all phases of construction. Such guying and bracing shall remain in place until the structure has reached adequate strength and is permanently braced.
- Steel fabricator shall provide $7/16"$ diameter holes $2'-0" \pm$ for wood anchorage where in contact.

GENERAL Description
11-6-81 Rev. Date
10-30-81 Date
Approved
Drawn

FIRST WISCONSIN
Brown Deer Road at
64th Street Branch

architects engineers
(414) 453-4554

Torke Wirth Pujara Ltd.
933 N. Mayfair Road Wauwatosa, WI 53226

Project Number
89214

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